General Notes

GN. GENERAL

GN.1 THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE A PORTION OF THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR AND SUBCONTRACTORS SHALL REFERENCE AND COORDINATE WITH ALL OTHER DISCIPLINES' DRAWINGS. ANY DISCREPANCIES OR OMISSIONS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER AND ARCHITECT.

GN.2 DESIGN CRITERIA:

A. CODES AND SPECIFICATIONS:

1. GENERAL BUILDING CODE:
NORTH CAROLINA STATE BUILDING CODE, 2018 EDITION.
(BASED ON INTERNATIONAL BUILDING CODE, 2015 EDITION).

2. DESIGN LOAD CRITERIA:
MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7.

3. CONCRETE:
BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318.

4. TIMBER:
NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, AMERICAN FOREST & PAPER ASSOCIATION/AMERICAN WOOD COUNCIL.

5. COLD-FORMED STEEL
NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AMERICAN IRON AND STEEL INSTITUTE.

6. MASONRY:
BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTURES, TMS 402/602.

B. DESIGN LOADS (PSF):

RISK CATEGORY------------------------------------II

1. DEAD LOADS:
ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE.

2. LIVE LOADS:
ROOF (REDUCIBLE)----------------------------------20

LIVE LOAD REDUCTIONS HAVE BEEN APPLIED IN ACCORDANCE WITH THE BUILDING CODE, UNLESS NOTED.

3. SNOW LOAD:
GROUND SNOW LOAD (Pg)---------------------------10.0
FLAT-ROOF SNOW LOAD (Pf)-------------------------7.0
SNOW EXPOSURE FACTOR (Ce)------------------------1.0
SNOW LOAD IMPORTANCE FACTOR (Is)-----------------1.0
THERMAL FACTOR (Ct)------------------------------1.0

4. WIND LOADS:
ULTIMATE DESIGN WIND SPEED, Vult-------------115 MPH
(3 – SECOND GUST)
NOMINAL DESIGN WIND SPEED, Vasd---------------89 MPH
(3 – SECOND GUST)
WIND EXPOSURE CATEGORY-----------------------------B
INTERNAL PRESSURE COEFFICIENT------------------±0.18

WALL COMPONENT AND CLADDING WIND PRESSURE-SEE DRAWINGS

5. SEISMIC LOADS:
SEISMIC IMPORTANCE FACTOR (Ie)-------------------1.0

MAPPED SPECTRAL RESPONSE ACCELERATIONS:
Ss---------------------------------------0.221
S1---------------------------------------0.098
ASSUMED SITE CLASS---------------------------------D
DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS:
SDs--------------------------------------0.235
SD1--------------------------------------0.156
SEISMIC DESIGN CATEGORY----------------------------C
BASIC SEISMIC-FORCE-RESISTING SYSTEM:
LIGHT-FRAME (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
SPECTRAL RESPONSE COEFFICIENT (Cs)-------------0.036
RESPONSE MODIFICATION FACTOR (R)-----------------6.5
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD

GN.3 CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION.

GN.4 SPECIAL INSPECTIONS/STRUCTURAL ENGINEER'S SITE VISITS:

A. SPECIAL INSPECTIONS ARE REQUIRED FOR THIS PROJECT. REFER TO DRAWINGS.

B. SITE VISITS BY THE STRUCTURAL ENGINEER'S OFFICE DO NOT REPLACE INSPECTIONS AND TESTING BY THE TESTING AGENCY OR SPECIAL INSPECTOR.

GN.5 SUBMITTALS:

A. REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. ALL SHOP DRAWINGS MUST BE REVIEWED AND "APPROVED" BY THE CONTRACTOR PRIOR TO SUBMITTAL.

B. ELECTRONIC SHOP DRAWING SUBMITTALS: SUBMIT ALL ELECTRONIC SHOP DRAWINGS IN .PDF FORMAT. REVIEWED SHOP DRAWINGS WILL BE RETURNED IN .PDF FORMAT. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED.

C. RESUBMITTED SHOP DRAWINGS: RESUBMITTED SHOP DRAWINGS SHALL HAVE ALL CHANGES SINCE THE PREVIOUS SUBMISSION IDENTIFIED BY CLOUDING OR OTHER CLEAR COMMUNICATION. RE-REVIEWED SHOP DRAWINGS WILL ONLY BE REVIEWED FOR IDENTIFIED CHANGES.

D. SHOP DRAWINGS: THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS. ITEMS MARKED (*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.

1. CONCRETE MIX DESIGNS
2. CONCRETE REINFORCING
3. COLD-FORMED METAL FRAMING
4. SHOP FABRICATED WOOD TRUSSES (*)
5. STRUCTURAL STEEL
6. MASONRY MORTAR MIX DESIGNS
7. MASONRY GROUT MIX DESIGNS
8. MASONRY REINFORCING

E. DESIGN CALCULATIONS: THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER'S RECORD, DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED FOR THE FOLLOWING ITEMS.

1. SHOP FABRICATED WOOD TRUSSES
2. COLD-FORMED METAL FRAMING

GN.6 ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS, UNLESS NOTED.

GN.7 THE CONTRACTOR IS RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

GN.8 CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED ROOFS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT LOADS DO NOT EXCEED THE DESIGN LIVE LOAD.

FD. FOUNDATION

FD.1 A GEOTECHNICAL ENGINEER, EMPLOYED BY THE CONTRACTOR, SHALL PROVIDE COMPACTED FILL REQUIREMENTS FOR THE BUILDING PAD AND REVIEW THE FOUNDATION BEARING SURFACE TO VERIFY THE BASIS OF DESIGN BEARING PRESSURE NOTED. DO NOT PLACE CONCRETE PRIOR TO GEOTECHNICAL ENGINEER'S APPROVAL.

FD.2 ASSUMED DESIGN BEARING PRESSURES:--------------------------------2000 PSF

FD.3 ALL FOUNDATION BEARING SURFACES SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE COMPLIANCE WITH PRESSURES NOTED. THE FINAL BEARING ELEVATIONS MAY VARY AS REQUIRED TO PROVIDE PROPER BEARING CAPACITY IN AN APPROVED BEARING STRATUM AS DETERMINED BY THE GEOTECHNICAL ENGINEER.

FD.4 FOOTINGS SHALL BE PLACED THE SAME DAY AS INSPECTION BY THE GEOTECHNICAL ENGINEER UNLESS EXTENDED TIME IS APPROVED BY THE GEOTECHNICAL ENGINEER.

FD.5 FOOTINGS SHALL BE NEATLY EXCAVATED WHERE POSSIBLE WITH SIDES AND TOP EDGES FREE OF LOOSE OR WET MATERIALS. WHERE NEAT EXCAVATION IS NOT POSSIBLE, FOOTING EXCAVATION SHALL BE FILLED WITH CONCRETE TO THE TOP OF FOOTING. THE BOTTOM EXCAVATION SHALL BE CLEAN AND DRY WITH ALL LOOSE MATERIAL REMOVED FOR AN ESSENTIALLY FLAT BEARING SURFACE. WHERE SOFT OR UNSUITABLE BEARING SURFACES ARE ENCOUNTERED, THE AREA SHALL BE UNDERCUT AS REQUIRED AND REPLACED WITH LEAN CONCRETE OR COMPACTED DENSE GRADED CRUSHED STONE AS DIRECTED BY THE GEOTECHNICAL ENGINEER.

FD.6 COMPACTED FILL SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT. EXCAVATED MATERIAL MAY BE USED AS BACKFILL MATERIAL WITH WRITTEN APPROVAL FROM THE GEOTECHNICAL ENGINEER STATING THAT SUCH MATERIAL IS SUITABLE AS BACKFILL AND INSTRUCTIONS ARE GIVEN FOR PROPER MOISTURE CONTENT AND COMPACTION.

FD.7 PROVIDE 4" OF COMPACTED GRANULAR FILL BENEATH ALL SLABS ON GRADE. PROVIDE 10 MIL VAPOR RETARDER BETWEEN BOTTOM OF SLAB AND TOP OF GRANULAR FILL.

FD.8 FOUNDATIONS SHALL BE CENTERED ABOUT COLUMN LINES, UNLESS NOTED.

CN. CONCRETE

CN.1 CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.

CN.2 MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS (PSI), TYPE OF CONCRETE, MAXIMUM W/C (WATER/CEMENTITIOUS MATERIALS RATIO), TOTAL AIR CONTENT, SLUMP AND CONCRETE USE

STRENGTH	TYPE	W/C	AIR	SLUMP	USE
3000	NORMAL WT.	0.57	4-6%	3" TO 5"	UNLESS NOTED

CN.3 REINFORCING BARS: ASTM A615 GRADE 60.

CN.4 WELDED WIRE REINFORCEMENT (WWR): ASTM A185. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2" OR 6".

CN.5 REINFORCING STEEL SHOWN IN SECTIONS AND DETAILS IS A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES AND GENERAL NOTES FOR ACTUAL REINFORCING REQUIRED.

CN.6 REINFORCING BAR PLACING ACCESSORIES TO BE INSTALLED IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE. WHERE CONCRETE IS EXPOSED IN FINISHED BUILDING, PROVIDE ACCESSORIES WITH RUSTPROOF LEGS.

CN.7 DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315. REINFORCEMENT SHALL NOT BE WELDED UNLESS NOTED OR APPROVED BY THE STRUCTURAL ENGINEER.

CN.8 SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.

CN.9 REINFORCING MARKED "CONTINUOUS" SHALL BE SPLICED WITH CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.

CN.10 CONCRETE COVERAGE OF REINFORCEMENT, UNLESS NOTED:

FOOTINGS------------------------------2" TOP & 3" BOTTOM & SIDES

WD. WOOD CONSTRUCTION

WD.1 WOOD FRAMING MEMBERS: VISUALLY GRADED DIMENSIONED #2 SOUTHERN PINE.

WD.2 SILL PLATES, SOLE PLATES AND TOP PLATES SHALL BE OF THE SAME SIZE AS THE STUDS TO WHICH THEY ARE CONNECTED. GRADE SHALL BE AS SPECIFIED ABOVE.

WD.3 ALL PRESSURE TREATED SOUTHERN PINE LUMBER SHALL BE PRESSURE TREATED WITH ALKALINE COPPER QUATERNARY (ACQ) IN ACCORDANCE WITH AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) STANDARD U1, COMMODITY SPECIFICATION A.

A. USE CATEGORIES:

1. UC2/INTERIOR DRY – SILL PLATES

B. ALL FASTENERS, NAILS AND OTHER METAL PRODUCTS USED WITH LUMBER PRESSURE TREATED WITH ACQ SHALL BE HOT-DIP GALVANIZED, STAINLESS STEEL OR AS RECOMMENDED BY THE ACQ MANUFACTURER. PRESSURE TREATED LUMBER SHALL NOT BE IN DIRECT CONTACT WITH ALUMINUM PRODUCTS.

WD.4 ALL MANUFACTURED WOOD FRAMING CONNECTORS TO BE BY SIMPSON STRONG-TIE COMPANY, INC. OR APPROVED EQUAL. ALL CONNECTORS SHALL BE FASTENED TO FRAMING MEMBERS FILLING THE REQUIRED NUMBER OF CONNECTOR HOLES WITH THE TYPE AND SIZE FASTENERS SPECIFIED BY THE MANUFACTURER.

WD.5 ROOF SHEATHING: 3/4" PLYWOOD, APA STRUCTURAL I RATED SHEATHING, EXPOSURE I. PANEL IDENTIFICATION INDEX 48/24. PROVIDE ONE H-CLIP AT ALL ADJOINING PANEL JOINTS MIDWAY BETWEEN SUPPORTS. LONG DIMENSION OF PANEL PERPENDICULAR TO SUPPORTS WITH JOINTS STAGGERED.

WD.6 ROOF SHEATHING NAILING, UNLESS NOTED: 10D HOT-DIPPED GALVANIZED COMMON NAILS AT 6 INCHES AT DIAPHRAGM BOUNDARIES, SHEAR WALLS, DRAG TRUSSES, AND DRAG STRUTS NOTED ON PLAN, 6 INCHES AT PANEL ENDS AND 12 INCHES AT INTERMEDIATE SUPPORTS.

WD.7 SHEAR WALL SHEATHING: 1/2" PLYWOOD, APA STRUCTURAL I RATED SHEATHING, EXPOSURE 1. PANEL IDENTIFICATION INDEX 32/16. LONG DIMENSION OF PANEL PERPENDICULAR TO STUDS (TRUSSES). ALL PLYWOOD EDGES BACKED WITH TWO-INCH NOMINAL OR WIDER FRAMING.

WD.8 SHEAR WALL SHEATHING NAILING, UNLESS NOTED: 10D HOT-DIPPED GALVANIZED COMMON NAILS AT 6 INCHES AT WALL BOUNDARIES, 6 INCHES AT ALL FOUR PANEL EDGES AND 12 INCHES AT INTERMEDIATE MEMBERS.

WD.9 LAMINATED VENEER LUMBER (LVL) BEAMS MANUFACTURED BY WEYERHAEUSER OR LOUISIANA PACIFIC CORPORATION OR APPROVED EQUAL SHALL MEET OR EXCEED THE REQUIREMENTS INDICATED BELOW:

BENDING (Fb)---2850 PSI
SHEAR PARALLEL TO GRAIN (Fv)--------------------------------285 PSI
COMPRESSION PERPENDICULAR TO GRAIN (Fc PER)-----------------750 PSI
MODULUS OF ELASTICITY (E)-------------------------------2,000,000 PSI
COMPRESSION PARALLEL TO GRAIN (Fc PAR)----------------------2510 PSI

WD.10 MULTI-PLY ENGINEERED LUMBER BEAMS, UNLESS NOTED OTHERWISE, ARE TO BE FASTENED TOGETHER WITH FASTENMASTER TRUSSLOK FASTENERS OR APPROVED EQUAL WITH A MINIMUM OF TWO ROWS OF FASTENERS AT 12 INCHES (STAGGERED) AND SPACED 3 INCHES FROM THE TOP AND BOTTOM OF BEAMS.

WD.11 REFER TO IBC TABLE 2304.10.1 FOR FASTENING REQUIREMENTS NOT SPECIFICALLY STATED IN DRAWINGS.

WD.12 BOLTS: ASTM A307 GRADE A W/ ASTM A653 HEX NUTS AND WASHERS.

WD.13 NAILS, WIRE BRADS, STAPLES: SHALL CONFORM TO ASTM F1667.

WD.14 POWER DRIVEN FASTENERS: SHALL CONFORM TO NER-272.

WD.15 WOOD SCREWS: SHALL CONFORM TO ASME B18.6.1.

WT. WOOD TRUSSES

WT.1 DESIGN, FABRICATE AND ERECT WOOD TRUSSES IN ACCORDANCE WITH THE "DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES" OF THE TRUSS PLATE INSTITUTE. ERECTION PLANS, TRUSS CALCULATIONS AND CONNECTION CALCULATIONS, DESIGNED BY THE CONTRACTOR, SHALL BE SUBMITTED FOR THE FILES OF THE STRUCTURAL ENGINEER. CALCULATIONS SHALL BEAR THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.

WT.2 THE WOOD TRUSS SYSTEM ENGINEER SHALL DESIGN THE COMPLETE TRUSS SYSTEM. THE TRUSS SYSTEM IS AN ASSEMBLAGE OF TRUSSES AND TRUSS GIRDERS, TOGETHER WITH ALL BRACING, CONNECTIONS AND OTHER STRUCTURAL ELEMENTS AND ALL SPACING AND LOCATIONAL CRITERIA THAT, IN COMBINATION, FUNCTION TO SUPPORT THE LOADS APPLICABLE TO THE STRUCTURE.

WT.3 TRUSS MANUFACTURER SHALL DESIGN FOR THE FOLLOWING SUPERIMPOSED LOADS:

A. ROOF:
1. TOP CHORD DEAD LOAD ---------------------------10 PSF
2. BOTTOM CHORD DEAD LOAD ------------------------10 PSF
3. TOP CHORD LIVE LOAD -------------SEE SECTION GN.2.B.2

WT.4 DESIGN ROOF TRUSSES TO RESIST THE WIND UPLIFT LOADING IN ACCORDANCE WITH THE COMPONENTS AND CLADDING TABLE IN THE DRAWINGS.

WT.5 IN ADDITION TO THE ABOVE LOADS, WOOD TRUSSES SHALL BE DESIGNED FOR CONCENTRATED LOADS HUNG FROM OR SUPPORTED ON TRUSSES. REFER TO MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND SPECIFICATIONS FOR LOADING INFORMATION AND LOCATION. LOADING AS REQUIRED BY OTHER SUBCONTRACTORS, SUCH AS FIRE PROTECTION, SHALL BE COORDINATED BY THE CONTRACTOR.

WT.6 ALL MANUFACTURED TRUSS HOLD-DOWNS TO BE BY SIMPSON STRONG-TIE COMPANY, INC. OR APPROVED EQUAL. ALL CONNECTORS SHALL BE FASTENED TO FRAMING MEMBERS FILLING THE REQUIRED NUMBER OF CONNECTOR HOLES WITH THE TYPE AND SIZE FASTENERS SPECIFIED BY THE MANUFACTURER.

WT.7 ALL TEMPORARY AND PERMANENT BRACING MEMBERS AND CONNECTIONS REQUIRED FOR WOOD TRUSSES SHALL BE DETAILED ON THE WOOD TRUSS MANUFACTURER'S ERECTION PLANS.

WT.8 TEMPORARY BRACING SHALL NOT IMPOSE ANY FORCE ON THE SUPPORTING STRUCTURE. PERMANENT BRACING FORCES SHALL BE TRANSFERRED TO THE ROOF OR FLOOR DIAPHRAGM BY THE BRACING DESIGN PROVIDED BY THE TRUSS MANUFACTURER.

CF. COLD-FORMED STEEL STRUCTURAL MEMBERS

CF.1 DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS(CFS) AND ACCESSORIES IS THE RESPONSIBILITY OF THE COLD-FORMED STEEL MANUFACTURER. THE CFS MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF THE CONNECTIONS BETWEEN MEMBERS AND THEIR CONNECTION TO THE BUILDINGS PRIMARY STRUCTURAL FRAME.

CF.2 ANY COLD-FORMED STEEL SIZES NOTED ARE FOR PRELIMINARY PRICING INFORMATION ONLY. THE COMPLETE DESIGN OF COLD-FORMED STEEL FRAMING SYSTEM AND PREPARATION OF ERECTION DRAWINGS ARE BY THE ENGINEER RESPONSIBLE FOR THEIR DESIGN.

CF.3 SUBMIT THE FOLLOWING:

A. PRODUCT DATA: FOR EACH TYPE OF COLD-FORMED STEEL PRODUCT AND ACCESSORY UTILIZED.

B. SHOP DRAWINGS: SHOW LAYOUT, SPACINGS, SIZES, THICKNESS, AND TYPES OF COLD-FORMED STEEL; FABRICATIONS; AND FASTENING AND ANCHORAGE DETAILS, INCLUDING MECHANICAL FASTENERS. SHOW REINFORCING CHANNELS, OPENING FRAMING, SUPPLEMENTAL FRAMING, STRAPPING, BRACING, BRIDGING, SPLICES, ACCESSORIES, CONNECTION DETAILS, AND ATTACHMENT TO ADJOINING WORK.

C. CALCULATIONS: COLD-FORMED STEEL DESIGN CALCULATIONS FOR THE FILES OF THE STRUCTURAL ENGINEER AND ARCHITECT. CALCULATIONS SHALL BEAR THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.

CF.4 PROVIDE COLD-FORMED STEEL CAPABLE OF WITHSTANDING DESIGN LOADS WITHIN LIMITS AND UNDER CONDITIONS INDICATED.

A. DESIGN LOADS AS INDICATED IN SECTION GN OF THESE GENERAL NOTES.

B. DEFLECTION LIMITS: DESIGN FRAMING SYSTEMS TO WITHSTAND DESIGN LOADS WITHOUT DEFLECTIONS GREATER THAN THE FOLLOWING:
1. EXTERIOR LOAD-BEARING WALL FRAMING: HORIZONTAL DEFLECTION OF 1/480 OF THE WALL HEIGHT.
2. INTERIOR LOAD-BEARING WALL FRAMING: HORIZONTAL DEFLECTION OF 1/360 OF THE WALL HEIGHT UNDER A HORIZONTAL LOAD OF 5 LB F/SQ. FT.

CF.5 VERTICAL STUDS SHALL BE 100% END BEARING.

CF.6 PROVIDE WALL BRACING, CONNECTION DETAILS, AND WINDOW HEADERS AS RECOMMENDED BY THE STUD MANUFACTURER FOR LOAD-BEARING STUDS.

CF.7 VERTICAL STUDS INTERRUPTED BY WALL OPENINGS SHALL BE LOCATED EQUALLY ON EACH SIDE OF THE OPENING. PROVIDE EVEN NUMBER OF FULL HEIGHT STUDS ON EACH SIDE OF OPENING. WELD STUD FLANGES TOGETHER WITH FILLET WELDS AT 6".

SS. STRUCTURAL STEEL

SS.1 FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

SS.2 STRUCTURAL STEEL AND STRUCTURAL STEEL CONNECTIONS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE:

W AND WT SHAPES	ASTM A992
STIFFENER PLATES, CAP PLATES, CONNECTION PLATES, AND ANGLES	ASTM A36
BASE PLATES	ASTM A572, GRADE 50
HOLLOW STRUCTURAL SECTIONS	ASTM A500, GRADE C
WELDED CONNECTIONS	E70XX ELECTRODES, MINIMUM SIZE FILLET WELD 3/16"
BOLTS	ASTM A325 OR A490
NUTS	ASTM A563
WASHERS	ASTM F436

SS.3 WHERE NO CAMBER IS INDICATED, BEAMS SHOULD BE ERECTED WITH NATURAL CAMBER ORIENTED UPWARD.

SS.4 HSS MEMBERS SHALL HAVE A 1/4" CLOSURE PLATE.

SS.5 STRUCTURAL STEEL MEMBERS SHALL NOT BE CUT, SPLICED, OR MODIFIED IN THE FIELD UNLESS NOTED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.

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DRAWN BY: LBYD

PANDA PROJECT #: D24956
PANDA STORE #: TBD
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GENERAL NOTES

S100

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General Notes (Continued)

MA. MASONRY

MA.1 MASONRY CONSTRUCTION SHALL CONFORM TO TMS 402/ACI 530/ASCE 5 AND TMS 602/ACI 530.1/ASCE 6 SPECIFICATIONS.

MA.2 CONCRETE MASONRY UNITS (CMU) SHALL BE LIGHTWEIGHT (DENSITY = 105 PCF), CONFORMING TO ASTM C90, UNLESS NOTED.

MA.3 COMPRESSIVE STRENGTH OF MASONRY (F'm): 2000 PSI AT 28 DAYS.

MA.4 GROUT SHALL CONFORM TO ASTM C476 WITH COMPRESSIVE STRENGTH (F'g) OF 2500 PSI AT 28 DAYS. GROUT SHALL BE PLACED ACCORDING TO TMS 602/ACI 530.1/ASCE 6 SECTION 3.5.

MA.5 MORTAR SHALL CONFORM TO ASTM C270, TYPE S OR M FOR TYPICAL CONDITIONS, TYPE M FOR BASEMENT AND RETAINING WALLS.

MA.6 ALL MASONRY SHALL BE RUNNING BOND, UNLESS NOTED.

MA.7 ALL BLOCK CELLS AND CAVITIES BELOW GRADE SHALL BE FILLED WITH CONCRETE OR GROUT.

MA.8 SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY CONTROL JOINTS AND OPENINGS.

MA.9 REINFORCING BARS: ASTM A615 GRADE 60. LAP REINFORCING BARS ACCORDING TO TYPICAL DETAILS.

MA.10 WHEN REINFORCING BARS ARE SPECIFIED, PROVIDE AT EACH SIDE OF CONTROL JOINTS, OPENINGS AND WALL ENDS ACCORDING TO TYPICAL DETAILS. REINFORCING BARS TO BE CENTERED IN WALL, UNLESS NOTED.

MA.11 CONDUIT, PIPING, AND SLEEVES OF ANY MATERIAL TO BE EMBEDDED IN MASONRY SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS:

A. CONDUIT, PIPING, AND SLEEVES OF ALUMINUM SHALL NOT BE EMBEDDED IN MASONRY.

B. CONDUIT, PIPING, AND SLEEVES SHALL NOT PASS THROUGH JAMBS, LINTELS, BOND BEAMS, OR SHEAR WALLS WITHOUT APPROVAL BY THE STRUCTURAL ENGINEER.

C. REINFORCING SHALL NOT BE CUT, BENT, OR DISPLACED FOR PLACEMENT OF CONDUIT, PIPING, AND SLEEVES.

D. CONDUIT, PIPING, AND SLEEVES SHALL BE NO CLOSER THAN 3 DIAMETERS ON CENTER. MINIMUM SPACING OF DIFFERENT DIAMETERS SHALL BE DETERMINED USING THE LARGER DIAMETER.

MA.12 TEMPORARY BRACING OF CMU WALLS IS THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL REMAIN IN PLACE UNTIL PERMANENT RESTRAINT IS PROVIDED.

Symbol Legend

SYMBOL	DESCRIPTION
	EXTERIOR/INTERIOR ELEVATION
	MOMENT CONNECTION
	BEAM SPLICE
	LEVEL CALLOUT
TOF -2'-0" (T.O.P., TODP, TOF, TOGB, TOPC, TOS, TOW)	SPOT ELEVATION
(n = DEPTH IN INCHES)	SLAB RECESS
	REVISION NUMBER

Abbreviations

ABOVE FINISH FLOOR - AFF
ADDITIONAL - ADDNL
ADJACENT - ADJ
AIR CONDITIONER - A/C
AIR HANDLING UNIT - AHU
ALTERNATE - ALT
ANCHOR - ANC
ANCHOR ROD - AR
AND - &
APPROVED - APPRV
APPROXIMATE - APPROX
ARCHITECTURAL - ARCH.
ARCHITECTURALLY EXPOSED STRUCTURAL STEEL - AESS
AT (WHEN INDICATING SPACING ONLY) - @

BACK TO BACK - B TO B
BALANCE - BAL
BASEMENT - BSMT
BASE PLATE - BSPL
BEAM - BM
BEARING - BRG
BELOW FINISH FLOOR - BFF
BETWEEN - BTWN
BLOCK - BLK
BLOCKING - BLKG
BOTTOM - BOT
BOTTOM OF - B.O.
BOTTOM CHORD EXTENSION - BCX
BRICK - BRK
BRIDGING - BRDG
BUILDING - BLDG

CAST IN PLACE - CIP
CENTER - CTR
CENTERLINE - CL
CENTER TO CENTER - C TO C
CHANNEL - C
CLEAR OR CLEARANCE - CLR
COLUMN - COL
COMPLETE JOINT PENETRATION - CJP
COMPRESSION - COMP
CONCRETE - CONC
CONCRETE MASONRY UNIT - CMU
CONNECTION(S) - CONN(S)
CONTINUOUS - CONT
CONTRACTOR - CONTR
CONSTRUCTION - CONST
CONTROL JOINT - CJ
CORNER - COR
COORDINATE - COORD
COVER PLATE - COV PL

DEGREE - DEG OR °
DEGREE CELSIUS - C°
DEGREE FAHRENHEIT - F°
DETAIL - DET
DEAD LOAD - DL
DIAGONAL - DIAG
DIAMETER - DIA or Ø
DIMENSION(S) - DIM(S)
DRAWING(S) - DWG(S)
DRILLED PIER - DP
DOUBLE - DBL
DOUBLE EXTRA STRONG - XXS
DOWEL(S) - DWL(S)
DOWN - DN

EACH - EA
EACH FACE - EF
EACH WAY - EW
EDGE OF DECK - EOD
EDGE OF SLAB - EOS
ELECTRICAL - ELEC
ELEVATION - EL
ELEVATOR - ELEV
EMBEDMENT - EMBED.
ENGINEER - ENGR
EQUAL - EQ
EQUIPMENT - EQUIP.
EXISTING - EXIST.
EXPANSION - EXP
EXPANSION ANCHOR - EXP ANC
EXPANSION JOINT - EJ
EXTENSION - EXTN
EXTERIOR - EXT
EXTRA STRONG - XS

FABRICATOR - FABR
FACE TO FACE - F TO F
FACE OF STUD - FOS
FAR SIDE - FS
FASTENER - FAS
FIELD VERIFY - FV
FINISH(ED) - FIN.
FINISHED FLOOR - FF
FLANGE - FLG
FLOOR - FLR
FLOOR DRAIN - FD
FOOT - FT
FOOTING - FTG
FOUNDATION - FDN
FRAMING - FRMG

GAGE OR GAUGE - GA
GALVANIZED - GALV
GENERAL - GEN
GENERAL CONTRACTOR - GC
GOVERNMENT - GOVT
GRADE - GR
GRADE BEAM - GB
GROUND - GRD

HARD ROCK - HD RK
HEADED STUD(S) - H STUD(S)
HEIGHT - HT
HIGH STRENGTH - HS
HOOK - HK
HORIZONTAL - HORZ

INFORMATION - INFO
INSIDE DIAMETER - ID
INSIDE FACE - I.F.
INTERIOR - INT
INTERMEDIATE - INTM

JOINT - JT
JOIST(S) - JST(S)
JOIST GIRDER - JG

KIPS (1000 LBS) - K
KIPS PER LINEAL FOOT - KLF
KIPS PER SQUARE INCH - KSI
KIPS PER SQUARE FOOT - KSF

LIGHTWEIGHT CONCRETE - LWT CONC
LIVE LOAD - LL
LONGITUDINAL - LONG.
LONG LEG HORIZONTAL - LLH
LONG LEG VERTICAL - LLV

MANUFACTURE(R) - MFR
MASONRY - MAS
MASONRY OPENING - MO
MATERIAL - MATL
MAXIMUM - MAX
MECHANICAL - MECH
METAL - MTL.
MIDDLE - MID
MINIMUM - MIN
MISCELLANEOUS - MISC
MOMENT - M
MOMENT CONNECTION(S) - MC

NEAR FACE - NF
NEAR SIDE - NS
NOMINAL - NOM
NOT IN CONTRACT - NIC
NOT TO SCALE - NTS
NUMBER - NO. or #

ON CENTER - OC
OPENING(S) - OPNG(S)
OPPOSITE - OPP
OUTSIDE FACE - O.F.
OUTSIDE DIAMETER - OD
OUTSTANDING LEG - OSL

PARALLEL - PAR.
PARTITION(S) - PARTN(S)
PENETRATION - PEN
PERMANENT - PERM
PERPENDICULAR - PERP
PLATE - PL
PLUMBING - PLBG
PNEUMATIC - PNEU
POST-TENSION - PT
POUNDS - LBS
POUNDS PER LINEAL FOOT - PLF
POUNDS PER SQUARE INCH - PSI
POUNDS PER SQUARE FT - PSF
POUNDS PER CUBIC YARD - PCY
POUNDS PER CUBIC INCH - PCI
PRECAST CONCRETE - PC
PREENGINEERED METAL BUILDING - PEMB
PREFABRICATED - PREFAB
PRELIMINARY - PRELIM
PRESSURE INJECTED FOOTING - PIF
PRESSURE TREATED - P.T.
PROJECTION - PROJ

RADIUS - R
REFERENCE - REF
REINFORCED CONC PIPE - RCP
REINFORCING - REINF
REQUIRED - REQD
RISER - RIS
ROOF - RF
ROOF DRAIN - RD
ROOF TOP UNIT - RTU
ROOM - RM
ROUND - RND
ROUGH OPENING - R.O.

SAWN LUMBER - SL
SCHEDULE - SCHED
SECTION - SECT
SHEAR - V
SHEET - SHT
SIMILAR - SIM
SPACE - SP
SPECIFICATION(S) - SPEC(S)
SPECIFIED - SPECD
SQUARE - SQ
STANDARD - STD
STEEL - STL
STIFFENER - STIFF.
STRAIGHT - STR
STIRRUPS - STIR.
STRUCTURE OR STRUCT'L - STRUCT
SYMMETRICAL - SYM
SUPPORT(S) - SUPT(S)

TEMPERATURE - TEMP
TENSION - T
THICK - THK
TONGUE AND GROOVE - T&G
TOP AND BOTTOM - T&B
TOP CHORD EXTENSION - TCX
TOP OF - T.O.
TOP OF FOOTING - TOF
TOP OF GRADE BEAM - TOGB
TOP OF JOIST - TOJ
TOP OF PIER - T.O.P.
TOP OF PILE CAP - TOPC
TOP OF STEEL - TOS
TOP OF WALL - TOW
TREAD - TR
TYPICAL - TYP

UNLESS NOTED - U.N.

VERTICAL - VERT

WATERPROOFING - WPFG
WATERSTOP - WS
WELDED WIRE REINFORCEMENT - WWR
WIND LOAD - WL
WIND MOMENT CONNECTION - WC
WINDOW - WDW
WITH - W/
WITHOUT - W/O
WOOD - WD
WORK POINT - WP
WEIGHT - WT
WIDE FLANGE - WF

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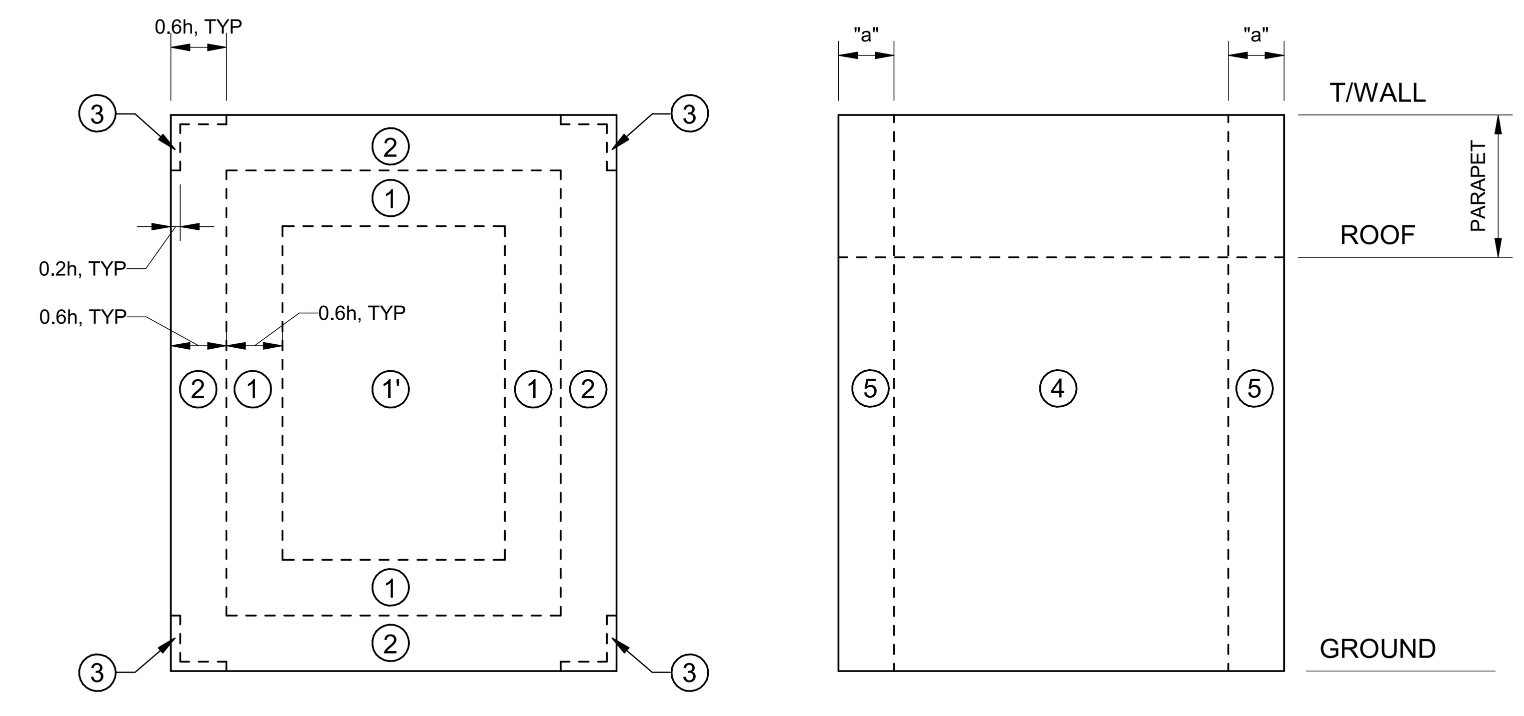

Roof Plan

Wall Elevation

Components and Cladding Wind Pressures

Components and Cladding Wind Pressures (Vult per ASCE 7-10)			
ZONE	EFFECTIVE WIND AREA (SQ FT)	MAX POSITIVE PRESSURE (PSF)	MAX NEGATIVE PRESSURE (PSF)
1' ROOF INTERIOR ZONE	10	16.0	-23.9
	20	16.0	-23.9
	50	16.0	-23.9
	100	16.0	-16.2
	>500	16.0	-16.0
1 ROOF INTERMEDIATE ZONE	10	16.0	-41.6
	20	16.0	-38.8
	50	16.0	-35.2
	100	16.0	-32.5
	≥500	16.0	-26.1
2 ROOF EDGE ZONE	10	23.9	-54.9
	20	22.8	-51.3
	50	21.4	-46.7
	100	20.4	-43.1
	≥500	17.9	-35.0
3 ROOF CORNER ZONE	10	23.9	-54.9
	20	22.8	-51.3
	50	21.4	-46.7
	100	20.4	-43.1
	≥500	17.9	-35.0
4 WALL INTERIOR ZONE	10	23.9	-25.9
	20	22.8	-24.8
	50	21.4	-23.4
	100	20.4	-22.4
	≥500	17.9	-19.9
5 WALL EDGE ZONE	10	23.9	-31.9
	20	22.8	-29.7
	50	21.4	-26.9
	100	20.4	-24.8
	≥500	17.9	-19.9

INTERNAL PRESSURE COEFFICIENT = ± 0.18
WIDTH OF WALL STRIP, a = 5'-5"
RELIABLE ROOF DEAD LOAD FOR UPLIFT = 6 PSF
WORST CASE ROOF DEAD LOAD = 25 PSF

Components and Cladding Parapet Wind Pressures (Vult per ASCE 7-10)		
ZONE	EFFECTIVE WIND AREA (SQ FT)	MAX (NET) PRESSURE (PSF)
4 INTERIOR ZONE	10	73.0
	20	68.2
	50	62.0
	100	57.3
5 EDGE ZONE	10	73.0
	20	68.2
	50	62.0
	100	57.3

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PANDA STORE #: TBD
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ARCHITECT LOGO

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GENERAL NOTES (CONTINUED) AND TYPICAL DETAILS

S101

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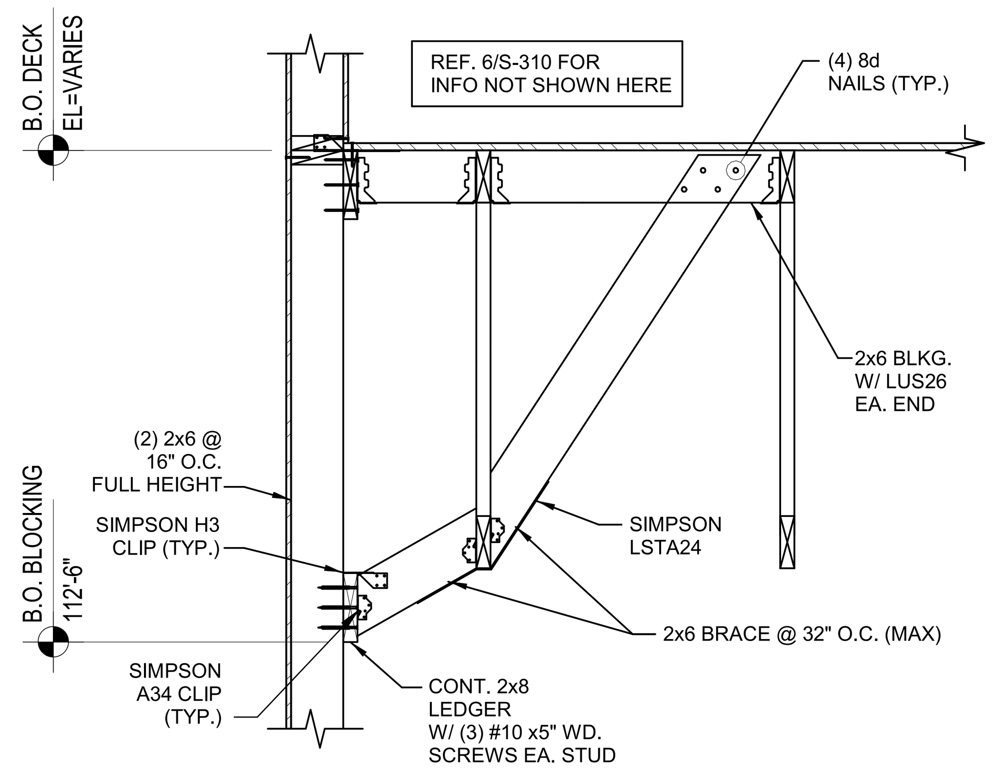
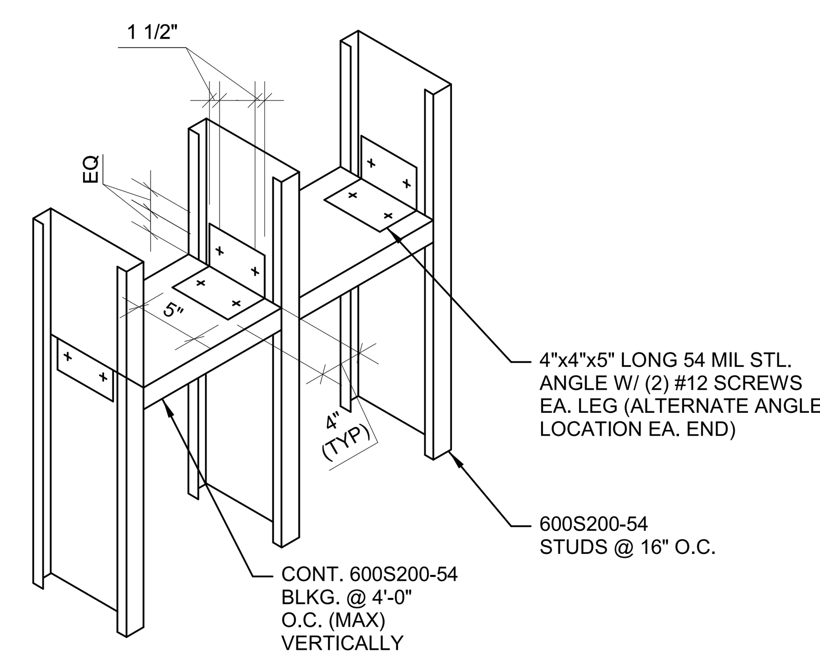
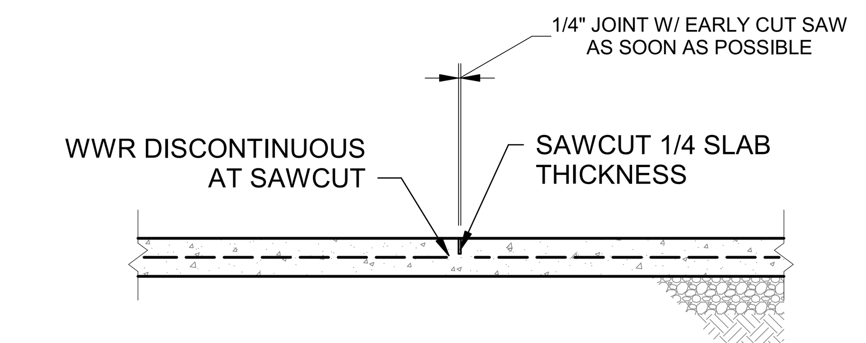


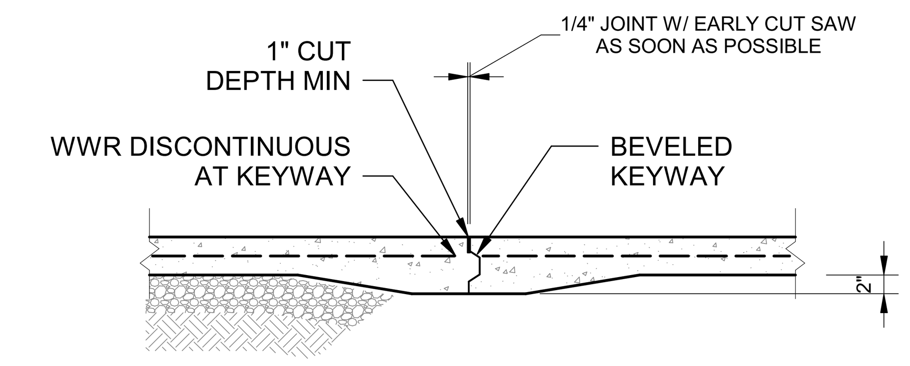
Alternate Storefront Brace Section



Typical Metal Stud Vertical Blocking



Control Joint

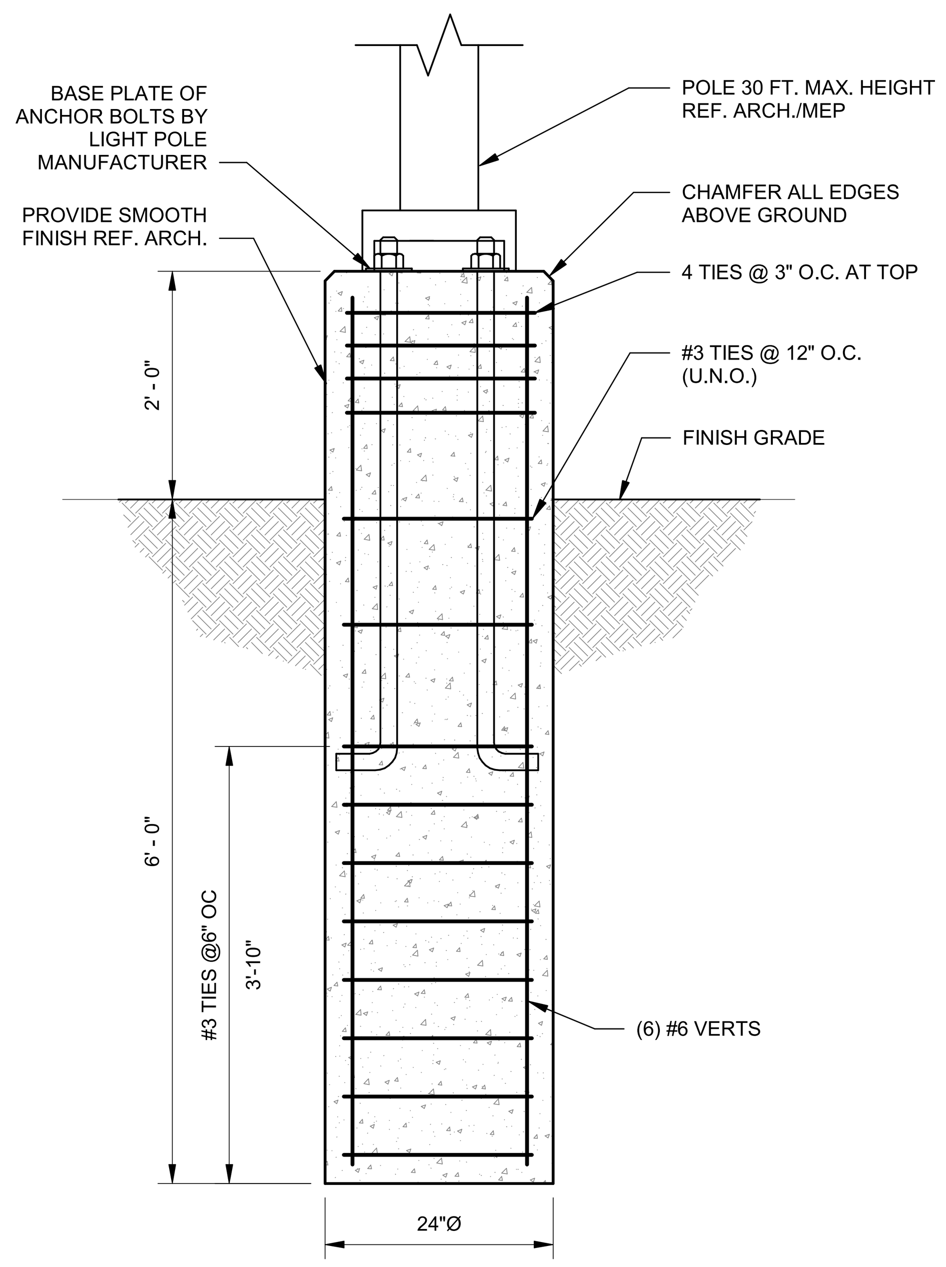


Construction Joint

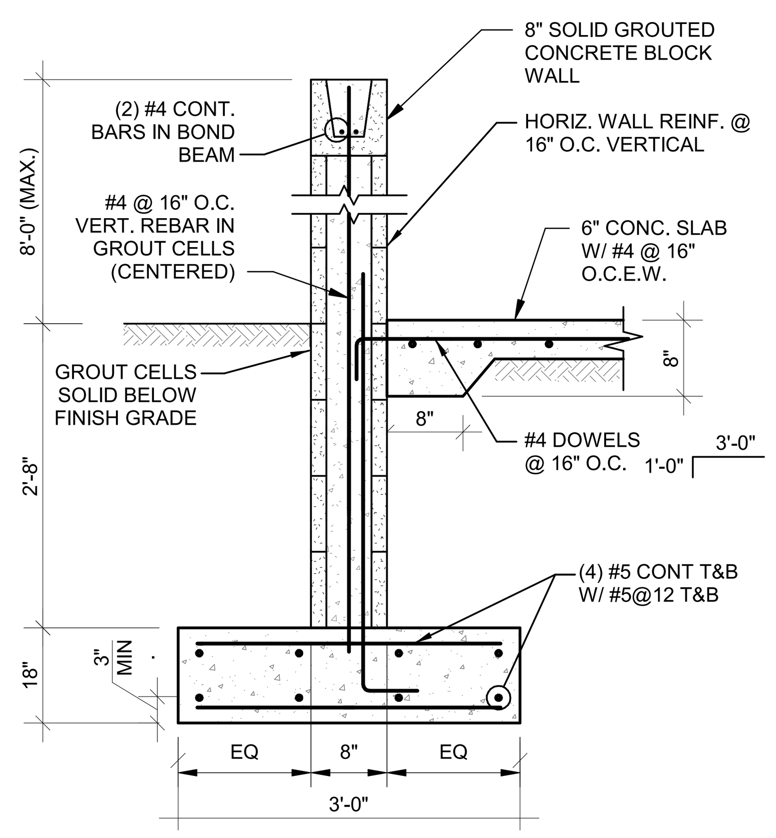
NOTES:
1. CONTROL JOINT FILLER SEMI-RIGID EPOXY.
2. SAWCUT TO TAKE PLACE WITHIN 4-12 HOURS OF FINISHING CONCRETE: 4 HOURS IN HOT WEATHER, 12 HOURS IN COLD WEATHER.

Slab Control Joint Details

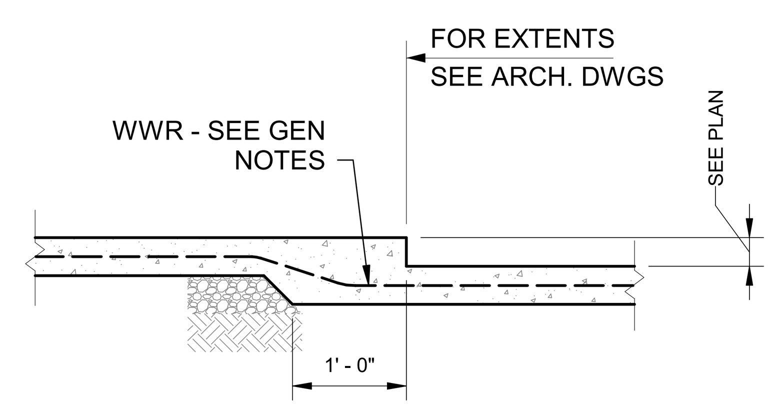
JOINT TYPE IS OPTIONAL



Typical Light Pole Section



Trash Enclosure Wall



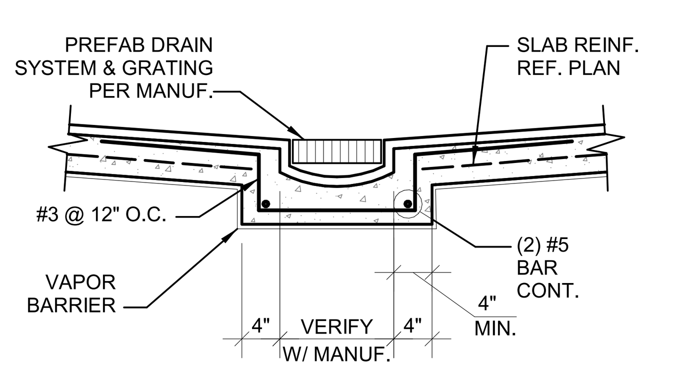
Depressed Slab on Grade Detail

SEE ARCH./MECH DRAWINGS FOR LOCATION AND SIZE

Tension Lap Splice Lengths

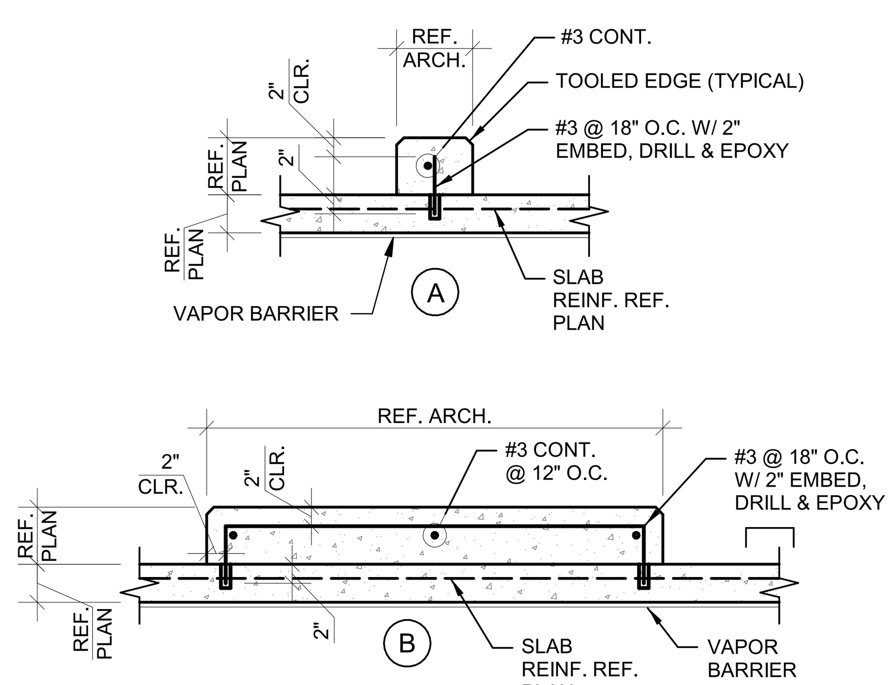
BAR SIZE	Concrete f_C = 3000	
	TOP BARS	OTHER BARS
#3	28"	22"
#4	37"	29"
#5	47"	36"
#6	56"	43"
#7	81"	63"
#8	93"	72"

1. TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF CONCRETE CAST BELOW THE REINFORCEMENT.



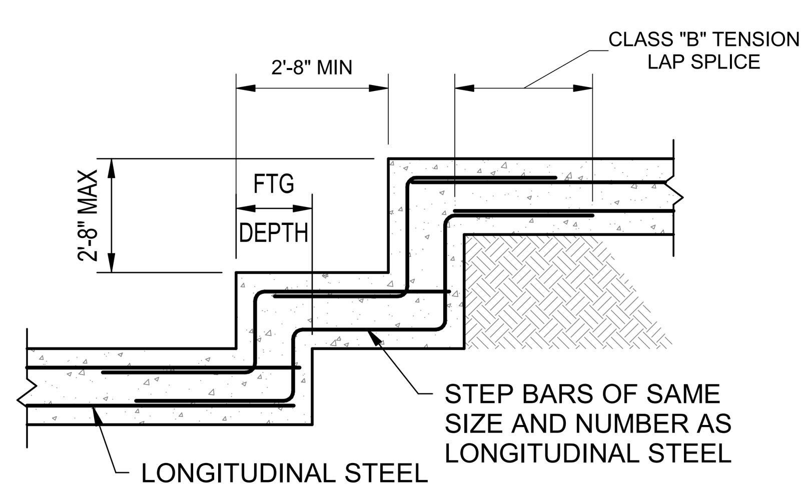
NOTE:
WHERE TRENCH CROSSES INTERIOR GRADE BEAM, BEND GRADE BEAM REINFORCEMENT UNDER DRAIN.

Typical Trench Drain

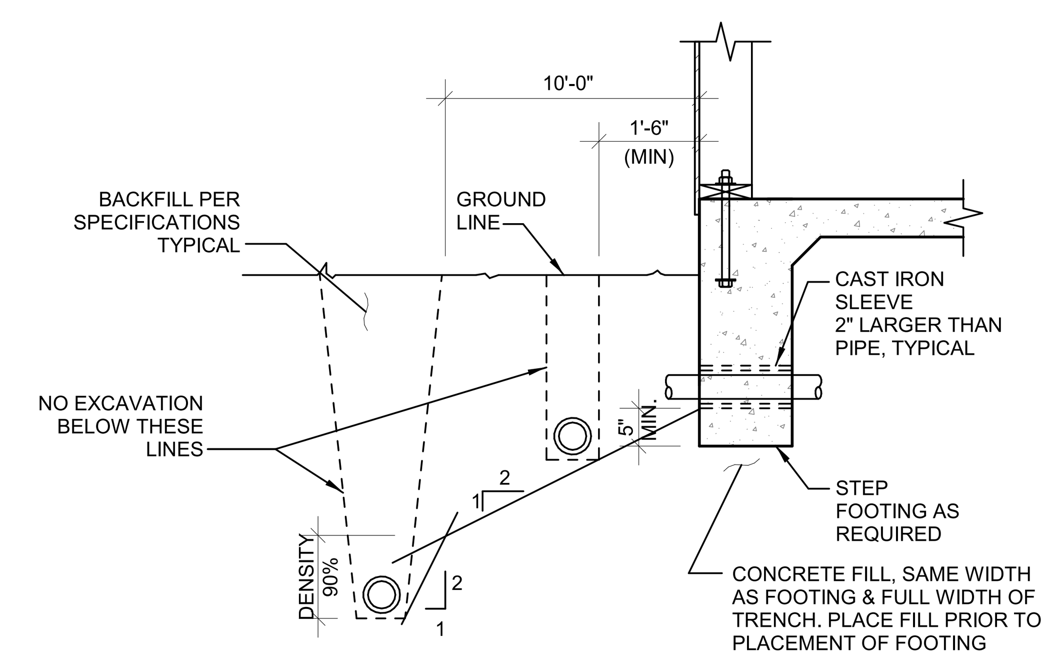


Typical Curb

NOTE:
SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND REQUIREMENTS.



Footing Step Detail



NOTE:FOR PIPES MORE THAN 3'-0" BELOW BOTTOM OF FOOTINGS, TRENCH MAY BE BACKFILLED WITH COMPACTED FILL PER SPECIFICATIONS. DO NOT TRENCH.

Typical Pipe & Trench Detail

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ARCHITECT LOGO

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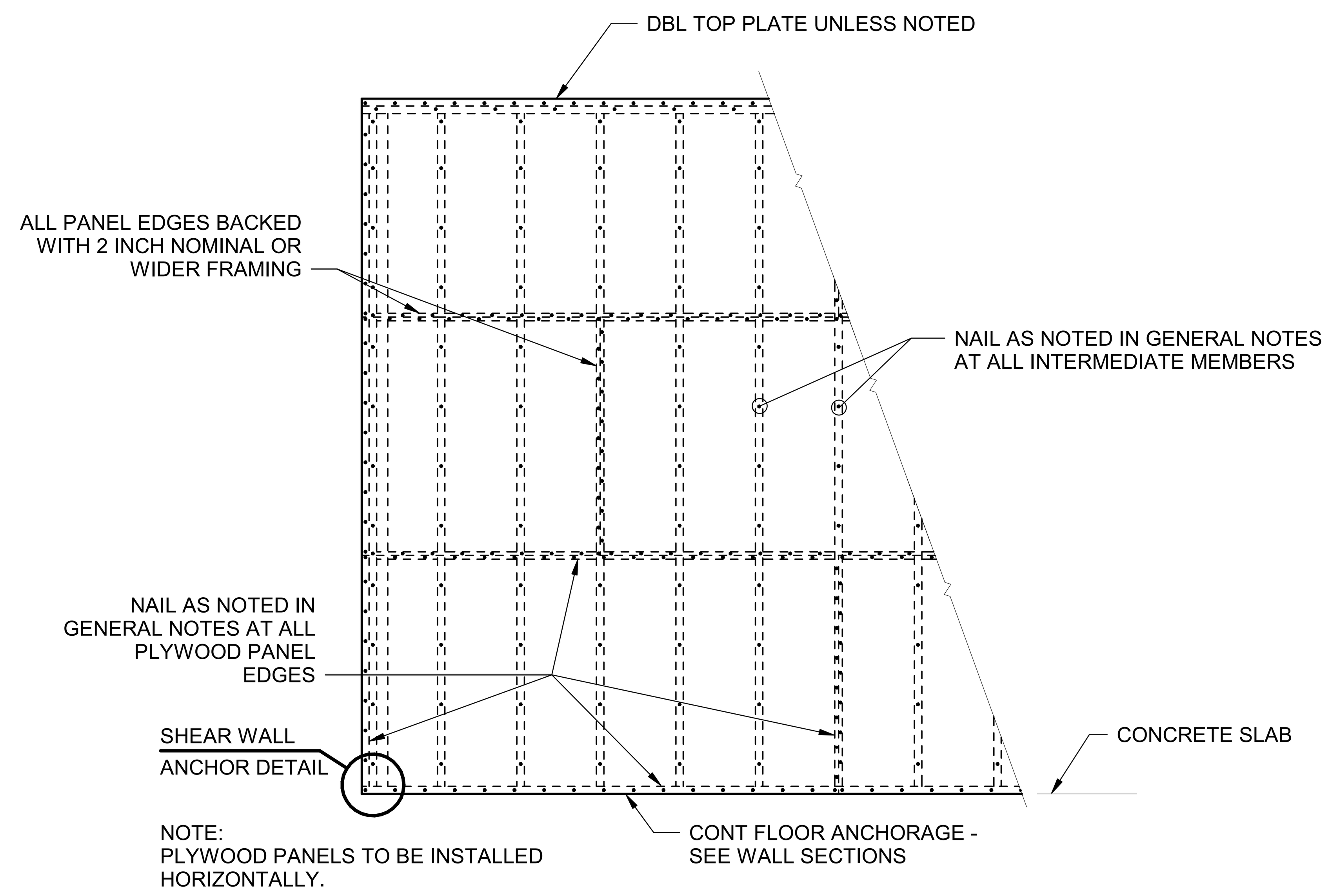
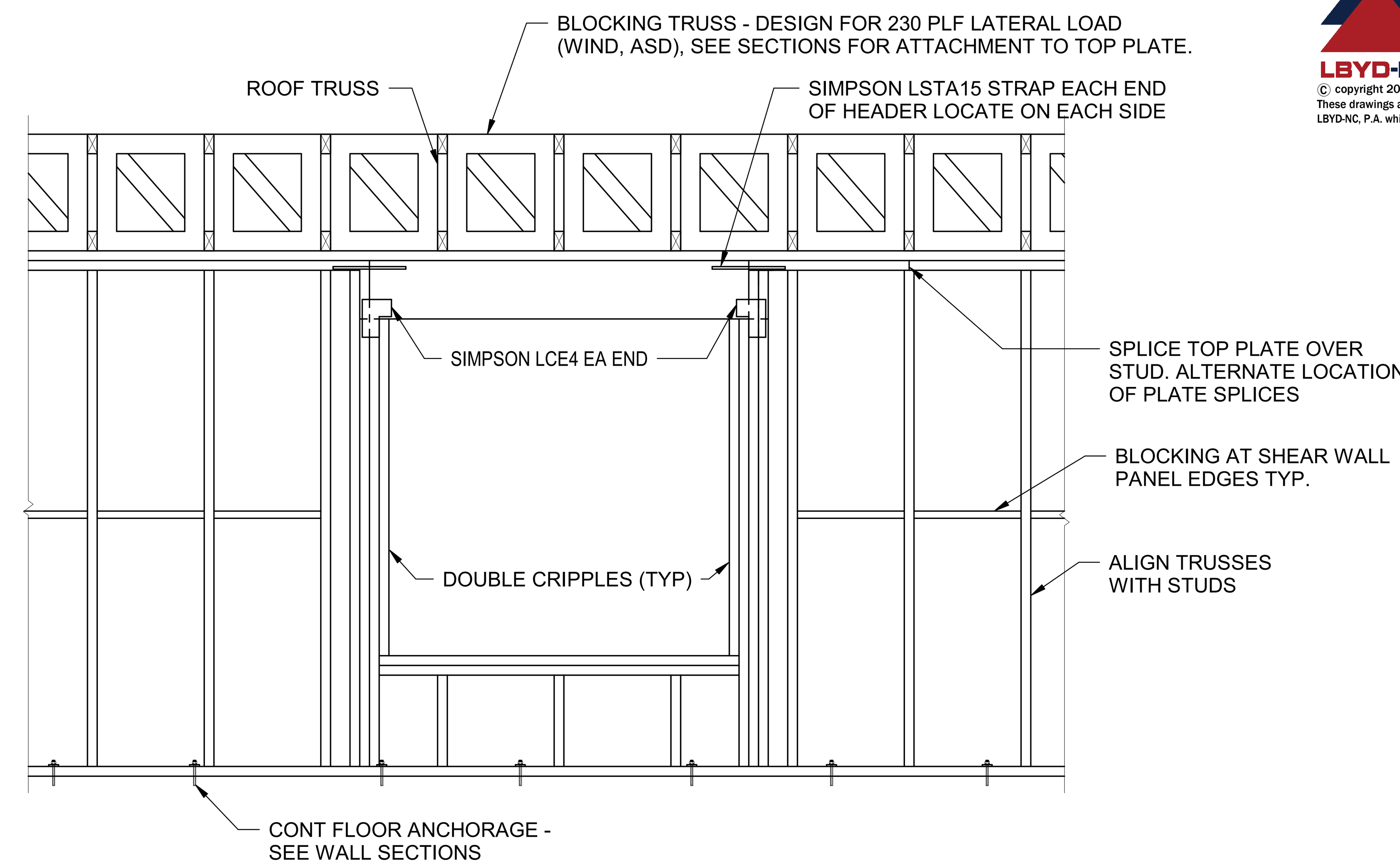
TYPICAL DETAILS

S102

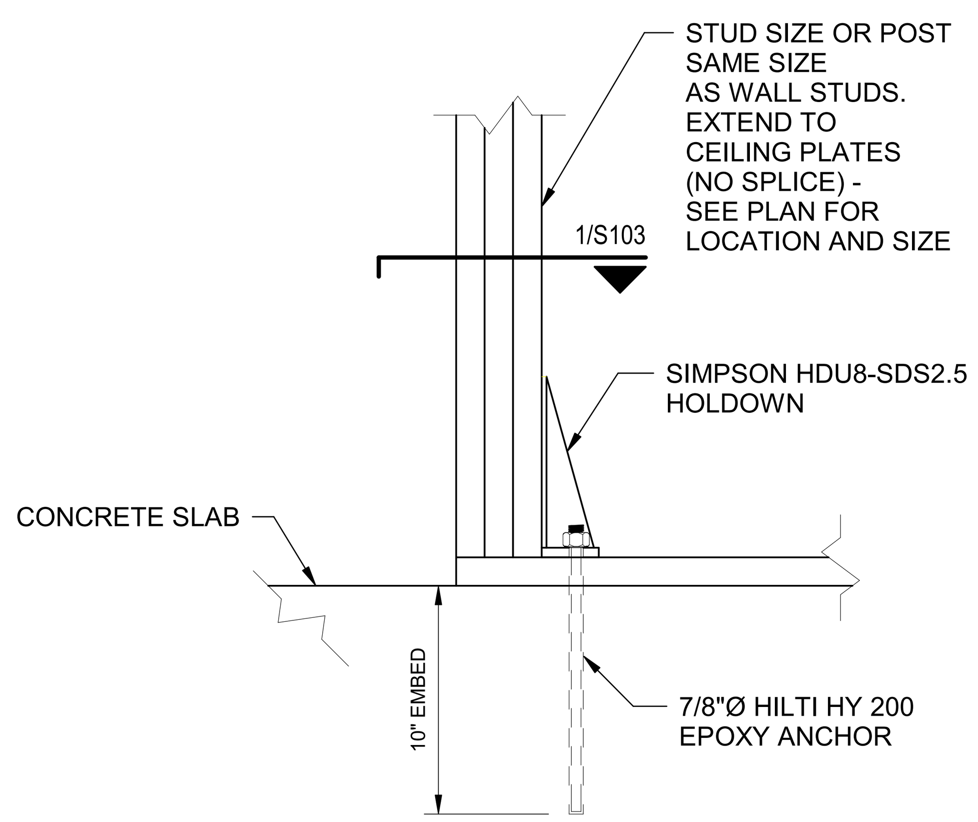


NOTE:
PLYWOOD PANELS TO BE INSTALLED HORIZONTALLY.

Exterior Shear Wall Panel Construction



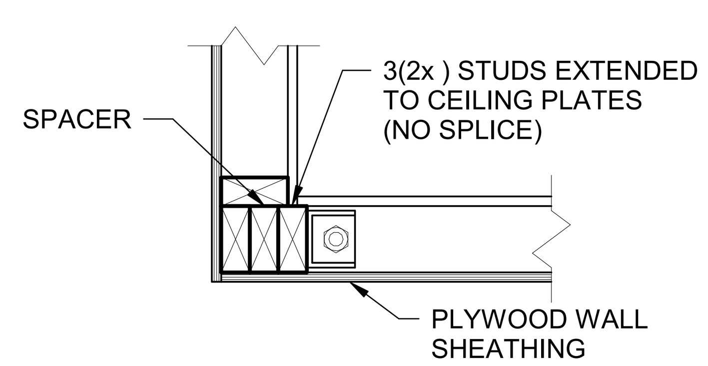
Shear Wall Elevation



Wood Shear Wall Anchor Detail

1 1/2"=1'-0"

SEE PLAN FOR LOCATIONS.

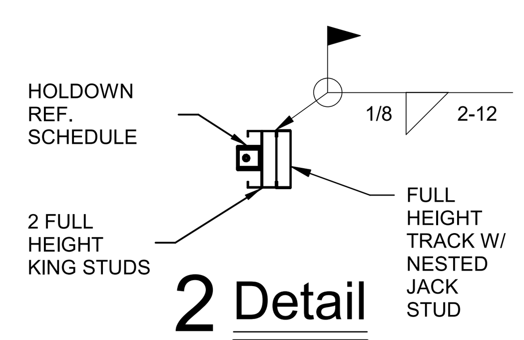


1 Detail

S103 1 1/2"=1'-0"

HOLDOWN	SCREW #	CAST IN PLACE REQUIREMENTS: ANCH. BOLT	CAST IN PLACE REQUIREMENTS: BOLT EMBED	EPOXY OPTION REQUIREMENTS: BOLT EMBED
S/HDU4	(6) #14 SCREWS	5/8"Ø	1'-0"	1'-0"

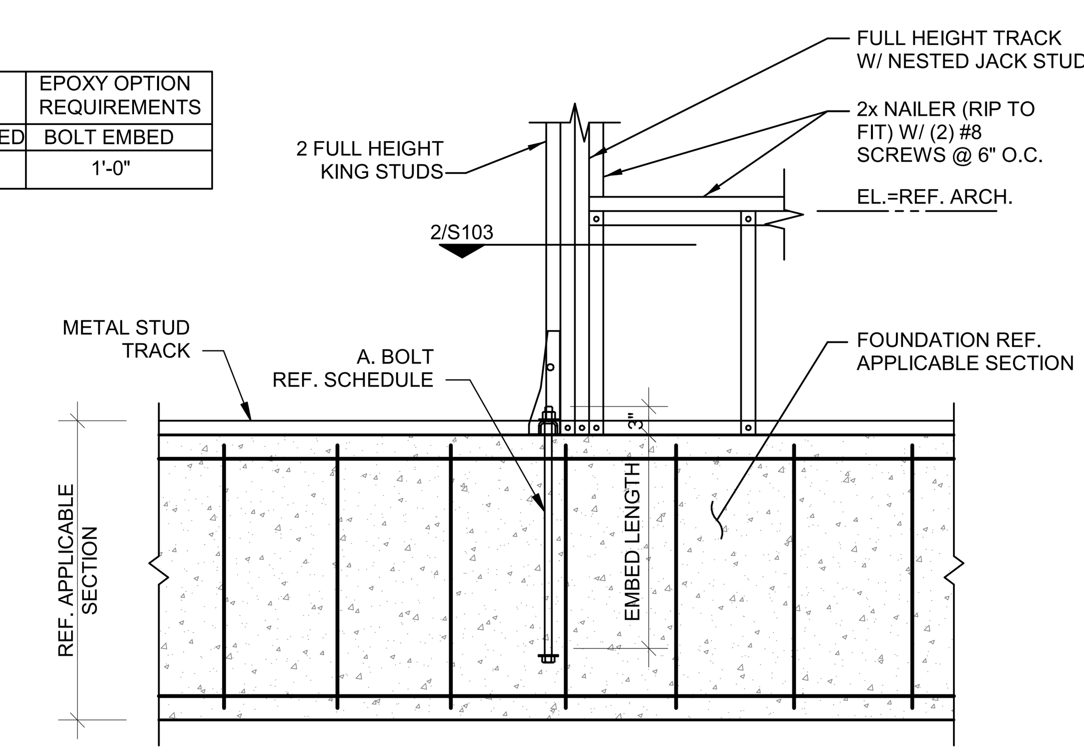
NOTE: ADD LENGTH TO BOLTS EXTENDING THROUGH CURBS.



2 Detail

S103 1 1/2"=1'-0"

NOTE: EPOXY OPTION IS BASED ON SIMPSON SET-XP DESIGN VALUES. EPOXY SUBSTITUTIONS MUST BE APPROVED BY STRUCTURAL ENGINEER OF RECORD BEFORE PLACEMENT.



Typical Holdown Detail - Cold-Formed Metal Stud

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TYPICAL DETAILS

S103

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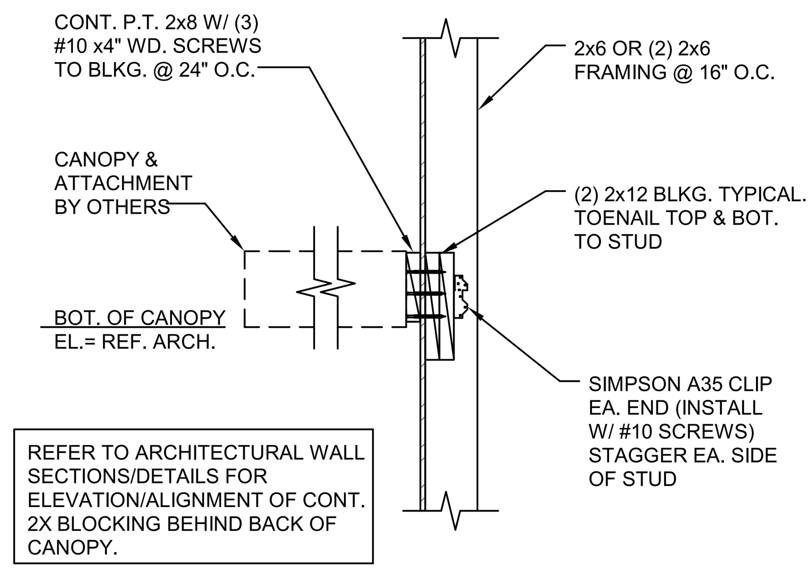
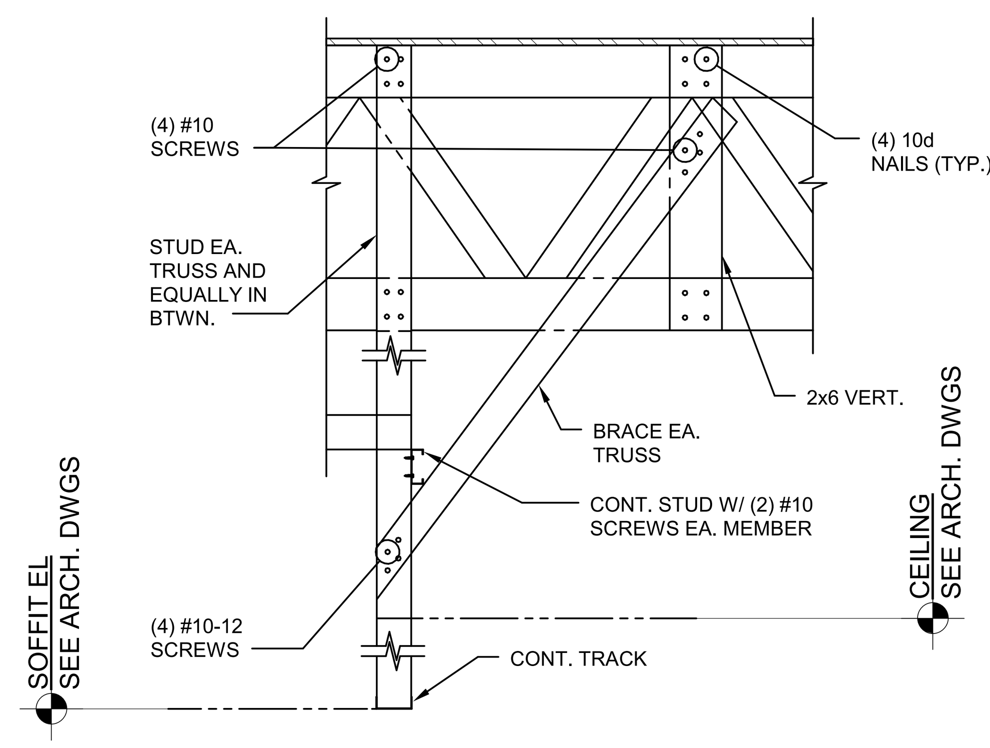
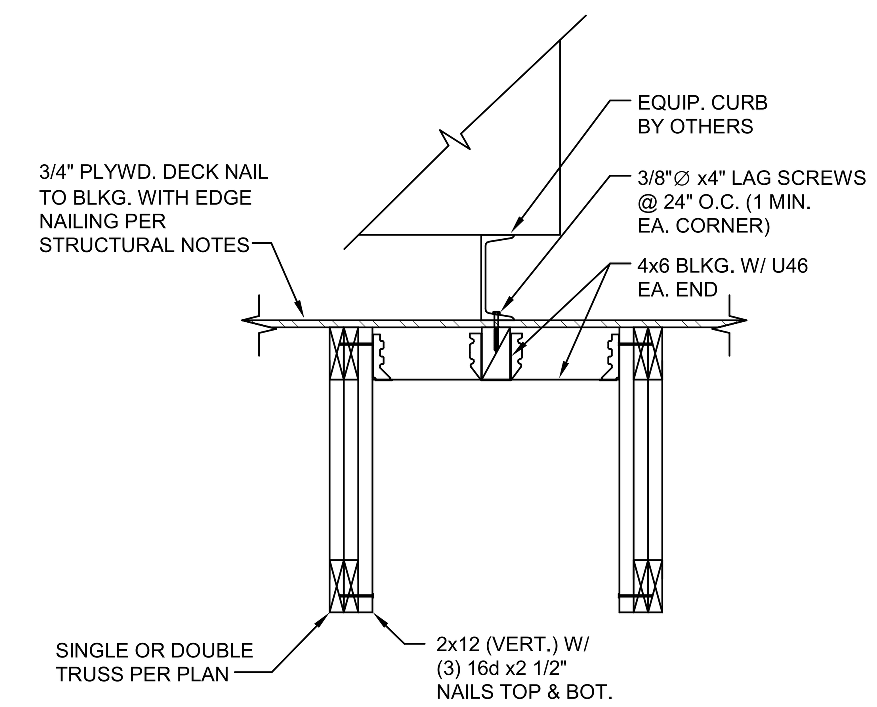


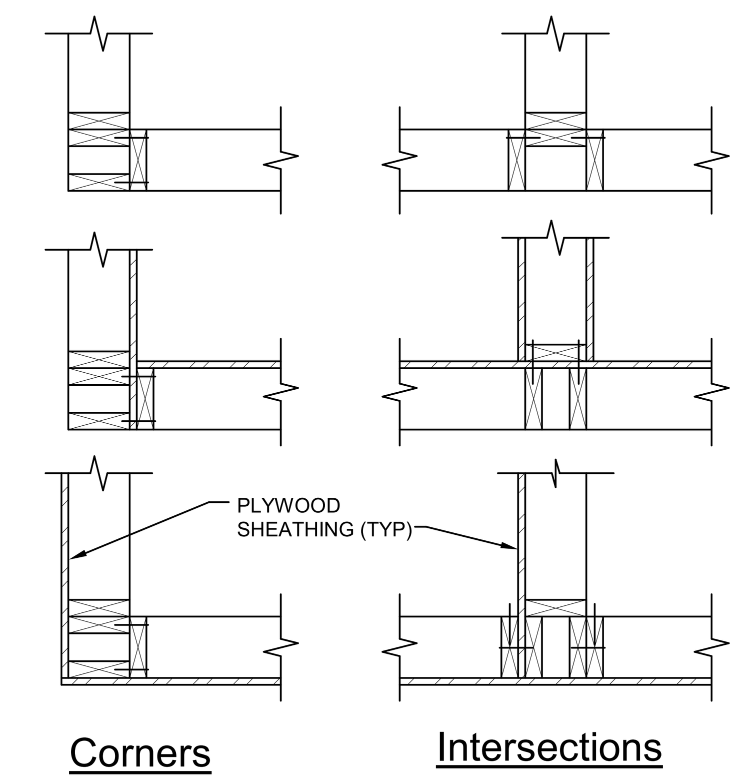
Typical Canopy Block



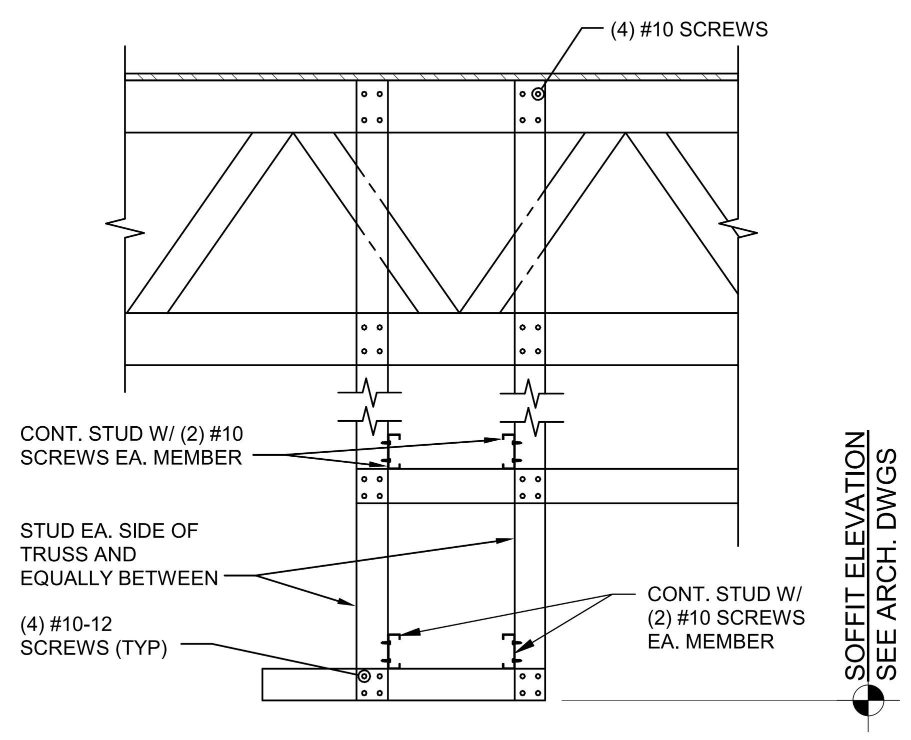
Soffit Detail at Service Counter



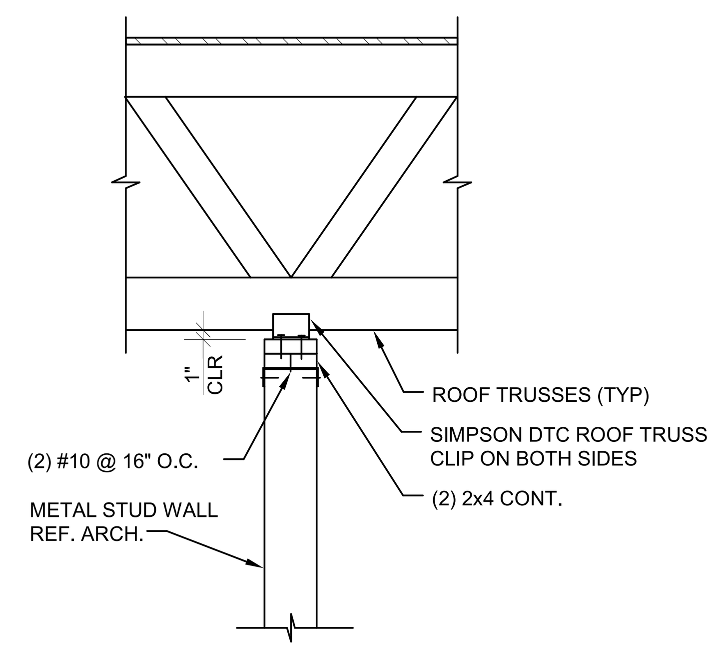
Typical Mechanical Support Unit



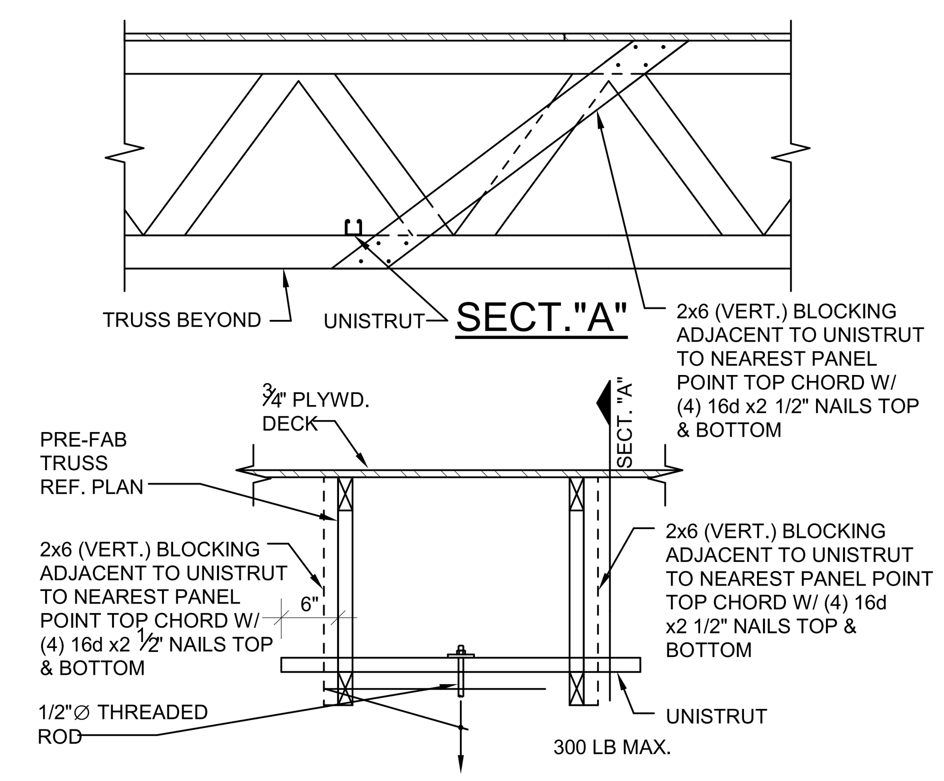
Typical Wall Framing



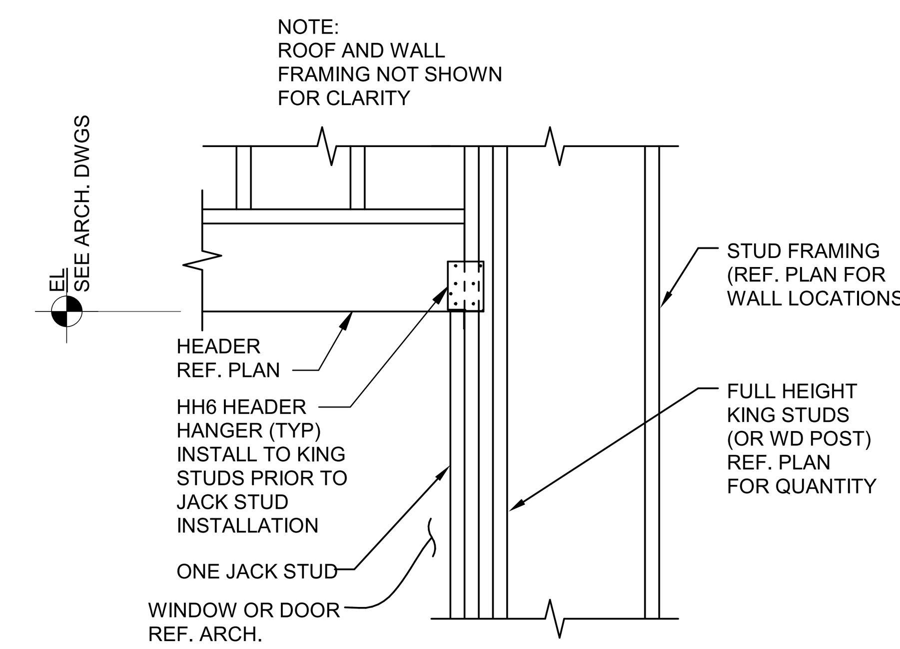
Soffit Detail at Serv Counter



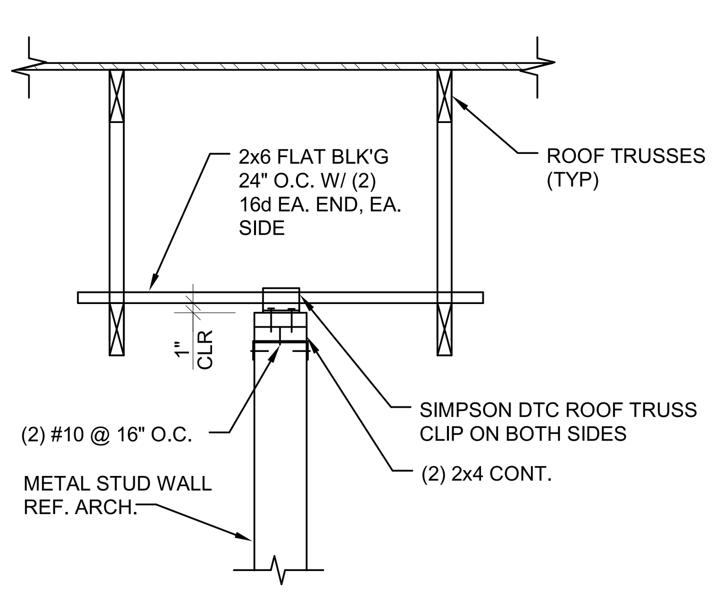
Support Perpendicular to Truss



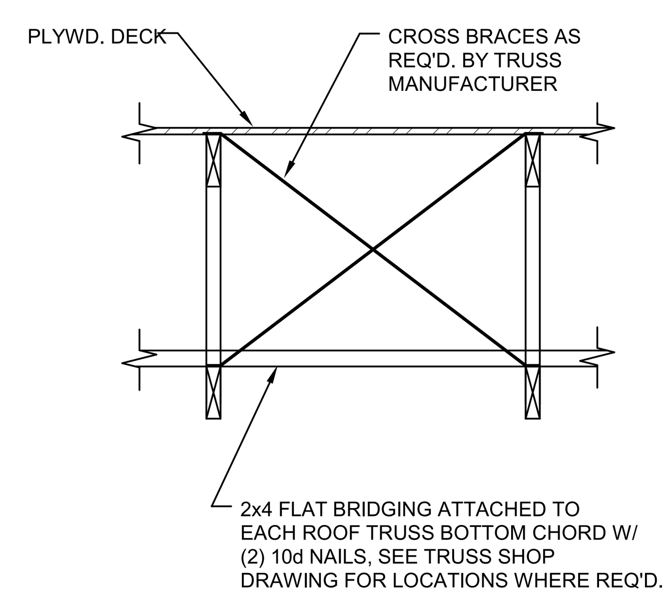
Typical Hanger Support



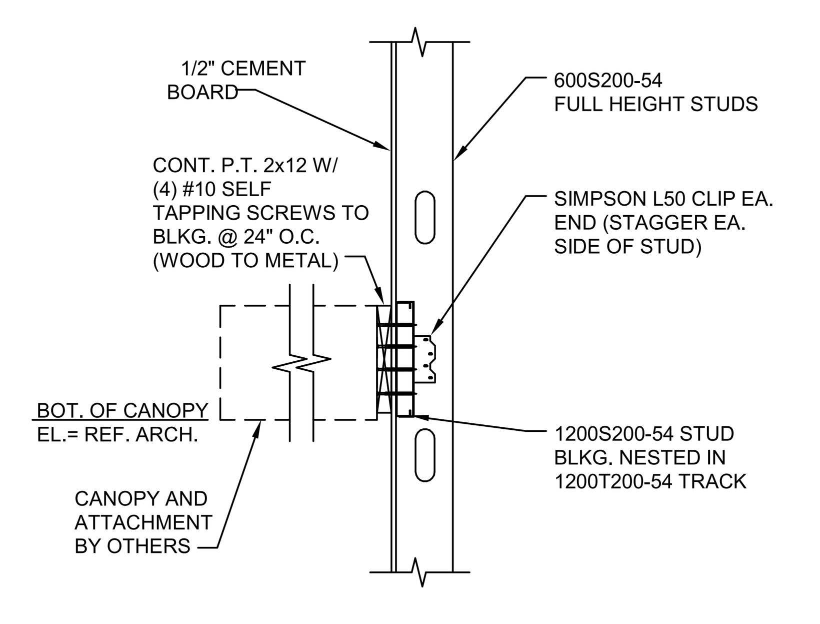
Typical Wood Header Framing



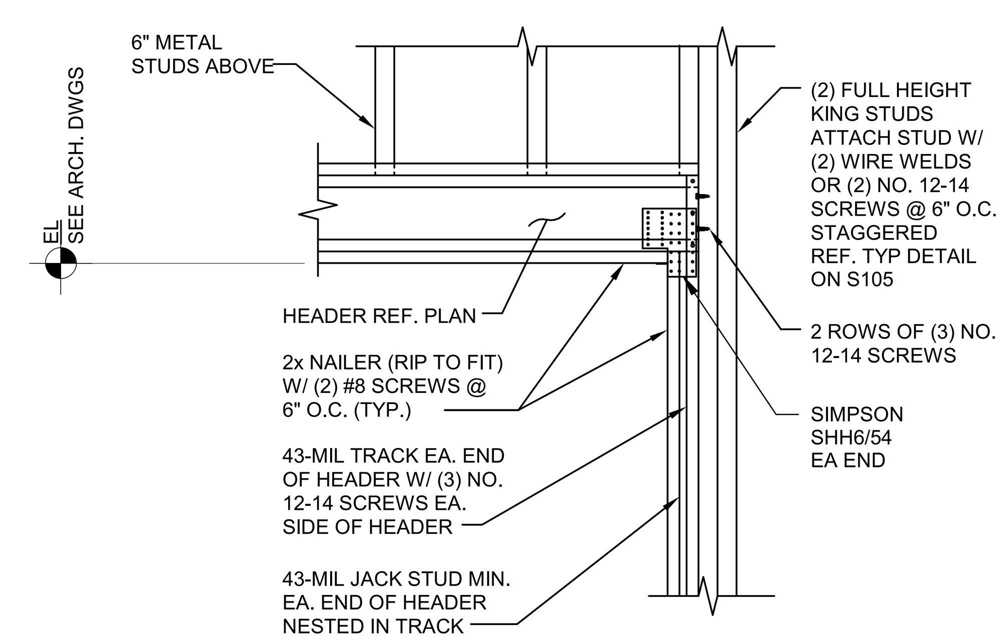
Support Parallel to Truss



Typical Bridging and Bracing



Small Canopy Blkg at Metal Stud



Typical Cold-Formed Metal Header Framing

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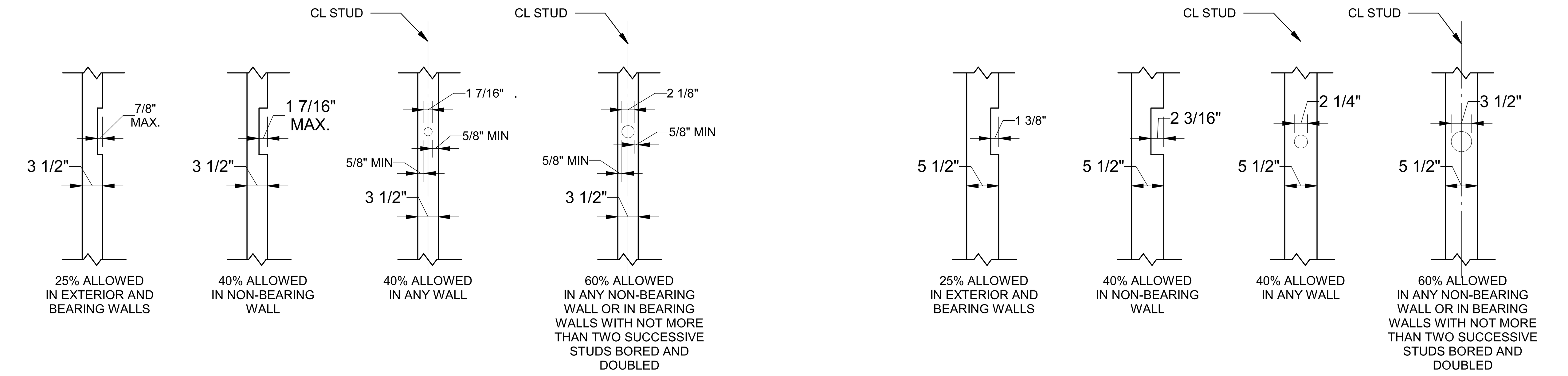
TYPICAL DETAILS

S104

Wood Fastening Schedule

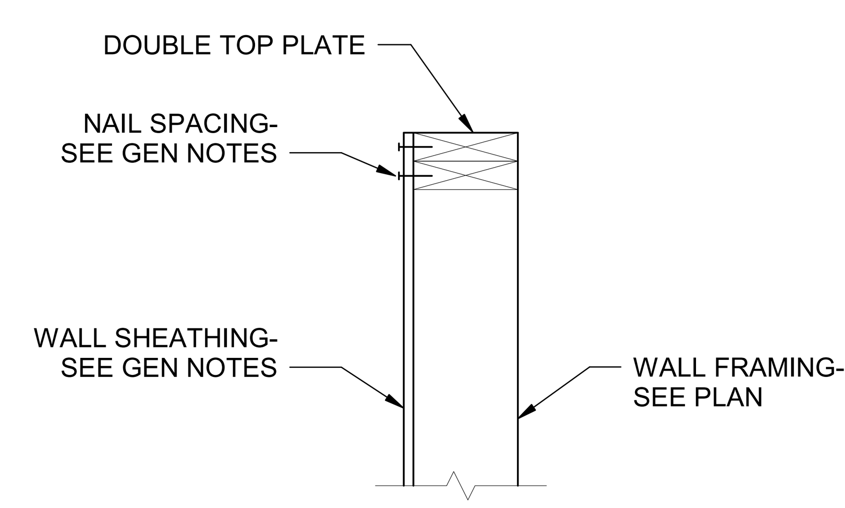
CONNECTION	FASTENING [a,m]	FASTENING
1. JOIST TO SILL OR GIRDER	3 - 8d COMMON 3 - 3"x0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL
2. BRIDGING TO JOIST	2 - 8d COMMON 2 - 3"x0.131" NAILS 2 - 3" 14 GAGE STAPLES	TOENAIL EACH END
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST	2 - 8d COMMON	FACE NAIL
4. WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST	3 - 8d COMMON	FACE NAIL
5. 2" SUBFLOOR TO JOIST OR GIRDER	2 - 16d COMMON	BLIND AND FACE NAIL
6. SOLE PLATE TO JOIST OR BLOCKING	16d AT 16" OC 3"x0.131" NAILS @ 8" OC 3" 14 GAGE STAPLES @ 12" OC	TYPICAL FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	3 - 16d @ 16" OC 4 - 3"x0.131" NAILS @ 16" 4 - 3" 14 GAGE STAPLES PER 16"	BRACED WALL PANELS
7. TOP PLATE TO STUD	1 - SIMPSON SOWC15600	TOENAIL
8. STUD TO SOLE PLATE	1 - SIMPSON SOWC15450	TOENAIL
9. DOUBLE STUDS	16d @ 24" OC 3"x0.131" NAILS @ 8" OC 3" 14 GAGE STAPLES 8" OC	FACE NAIL
10. DOUBLE TOP PLATES	16d @ 16" OC 3"x0.131" NAILS @ 12" OC 3" 14 GAGE STAPLES @ 12" OC	TYPICAL FACE NAIL
DOUBLE TOP PLATES	8 - 16d COMMON 12 - 3"x0.131" NAILS 12 - 3" 14 GAGE STAPLES	LAP SPLICE
11. BLOCKING BETWEEN JOIST OR RAFTERS TO TOP PLATE	3 - 8d COMMON 3 - 3"x0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL
12. RIM JOIST TO TOP PLATE	8d @ 6" OC 3"x0.131" NAILS @ 6' OC 3" 14 GAGE STAPLES @ 6' OC	TOENAIL
13. TOP PLATES, LAPS AND INTERSECTIONS	2 - 16d COMMON 3 - 3"x0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
14. CONTINUOUS HEADER, TWO PIECES	16d COMMON	16" OC ALONG EDGE
15. CEILING JOIST TO PLATE	3 - 8d COMMON 5 - 3"x0.131" NAILS 5 - 3" 14 GAGE STAPLES	TOENAIL
16. CONTINUOUS HEADER TO STUD	4 - 8d COMMON	TOENAIL
17. CEILING JOIST, LAP OVER PARTITIONS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	3 - 16d COMMON MINIMUM, TABLE 2308.10.4.1 4 - 3"x0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
18. CEILING JOIST TO PARALLEL RAFTERS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	3 - 16d COMMON MINIMUM, TABLE 2308.10.4.1 4 - 3"x0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
19. RAFTER TO PLATE (SEE SECTION 2308.10.1, TABLE 2308.10.1)	3 - 8d COMMON 3 - 3"x0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL
20. 1" DIAGONAL BRACE TO EACH STUD AND PLATE	2 - 8d COMMON 2 - 3"x0.131" NAILS 2 - 3" 14 GAGE STAPLES	FACE NAIL
21. 1"x8" SHEATHING TO EACH BEARING WALL	2 - 8d COMMON	FACE NAIL
22. WIDER THAN 1"x8" SHEATHING TO EACH BEARING	3 - 8d COMMON	FACE NAIL
23. BUILT-UP CORNER STUDS	16d COMMON 3"x0.131" NAILS 3" 14 GAGE STAPLES	24" OC 16" OC 16" OC
24. BUILT-UP GIRDER AND BEAMS	20d COMMON 32" OC 3"x0.131" NAIL @ 24" OC 3" 14 GAGE STAPLES @24" OC	FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
	2 - 20d COMMON 3 - 3"x0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL AT ENDS AND AT EACH SPLICE
25. 2" PLANKS	16d COMMON	AT EACH BEARING
26. COLLAR TIE TO RAFTER	3 - 10d COMMON 4 - 3"x0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
27. JACK RAFTER TO HIP	3 - 10d COMMON 4 - 3"x0.131" NAILS 4 - 3" 14 GAGE STAPLES	TOENAIL
	2 - 16d COMMON 3 - 3"x0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
28. ROOF RAFTER TO 2-BY RIDGE BEAM	2 - 16d COMMON 3 - 3"x0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOENAIL
	2 - 16d COMMON 3 - 3"x0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
29. JOIST TO BAND JOIST	3 - 16d COMMON 5 - 3"x0.131" NAILS 5 - 3" 14 GAGE STAPLES	FACE NAIL
30. LEDGER STRIP	3 - 16d COMMON 4 - 3"x0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL



2x4 Stud Walls

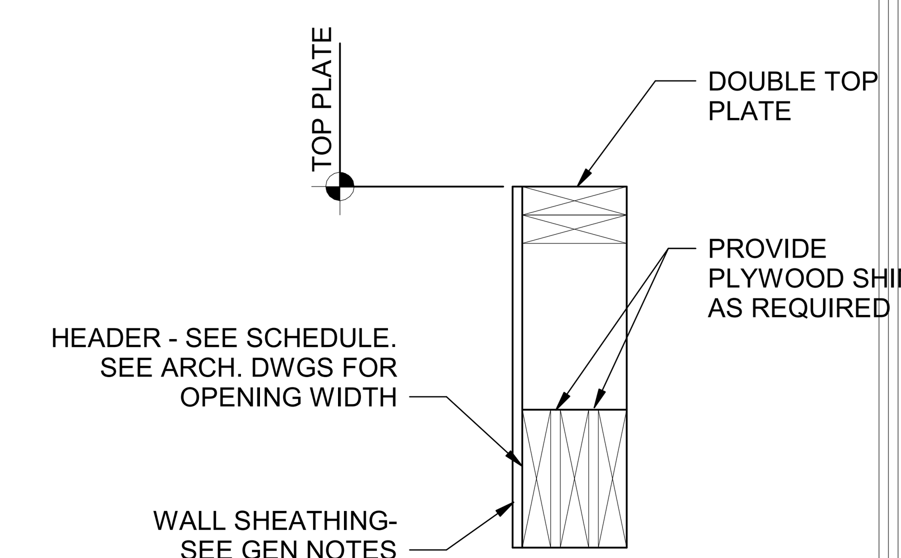
2x6 Stud Walls

Allowable Framing Notch Detail



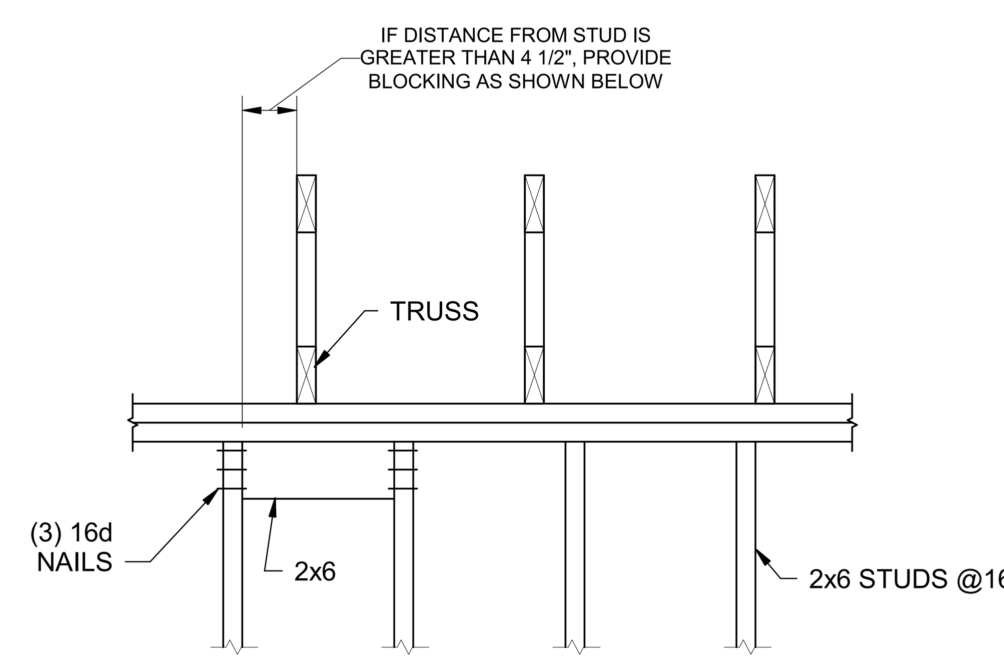
A Top Plate

NOTE:
SPLICE TOP PLATE W/ (10) 10d NAILS EA SIDE OF SPLICE.



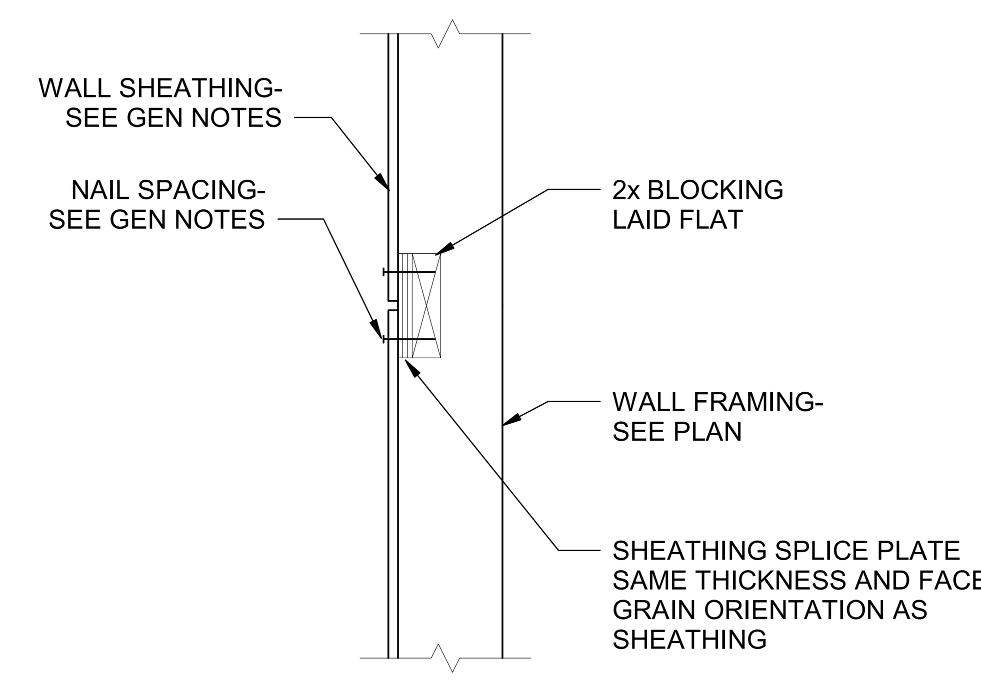
B Exterior Header

NOTE:
SPLICE TOP PLATE W/ (10) 10d NAILS EA SIDE OF SPLICE.



NOTE:
DETAIL APPLIES TO ROOF TRUSSES.

Truss Bearing Detail



C Splice Plate

Header Schedule

Clear Opening	Header	Material	Stud Pack	Notes
≤8'-0"	(3) 2x12	SL	3 EA SIDE	

NOTES:
1. HEADERS FOR OPENINGS LARGER THAN 8'-0" ARE INDICATED ON PLAN.

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ARCH PROJECT #: 239201

ARCHITECT LOGO

bdg architects

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PANDA HOME 2600
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Concord, NC 28027

TYPICAL DETAILS

S105

Special Inspection General Notes

SI.1 ALL SPECIAL INSPECTIONS SHALL BE PERFORMED IN CONFORMANCE WITH THE APPLICABLE IBC AND ITS REFERENCED SPECIFICATIONS.

SI.2 THE SPECIAL INSPECTOR SHALL BE EMPLOYED BY THE OWNER OR THE OWNER'S AGENT AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED.

SI.3 THE SPECIAL INSPECTOR SHALL BE QUALIFIED PER THE INTERNATIONAL BUILDING CODE AND SHALL BE EDUCATED IN THE TASKS REQUIRED TO CONDUCT, SUPERVISE, AND EVALUATE THE INSPECTIONS. THE SPECIAL INSPECTOR MUST ALSO BE OBJECTIVE, COMPETENT, AND HAVE ACCESS TO THE APPROPRIATE TESTING EQUIPMENT WHICH SHALL BE MAINTAINED AND PERIODICALLY CALIBRATED.

SI.4 SPECIAL INSPECTION AGENTS:

ATA: APPROVED TESTING AGENCY

GEOR: GEOTECHNICAL ENGINEER OF RECORD:
TBD

SEOR: STRUCTURAL ENGINEER:
LBYD ENGINEERS
880 MONTCLAIR ROAD, SUITE 600
BIRMINGHAM, AL 35213
(205) 251-4500

SI.5 THE SPECIAL INSPECTIONS SHALL BE PERFORMED IN ADDITION TO ANY OBSERVATIONS PERFORMED BY THE ENGINEER OF RECORD.

SI.6 THE SPECIAL INSPECTOR SHALL MAINTAIN RECORDS AND PROVIDE THE REQUIRED DOCUMENTATION AS PRESCRIBED IN THE INTERNATIONAL BUILDING CODE, INCLUDING THE SUBMITTAL OF REPORTS TO THE DESIGNER OF RECORD.

SI.7 THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION SCHEDULE WITH THE SPECIAL INSPECTOR TO ALLOW FOR SPECIAL INSPECTIONS.

SI.8 CONSTRUCTION WHICH REQUIRES SPECIAL INSPECTIONS SHALL BE MAINTAINED IN SUCH A STATE AS TO ALLOW ACCESS FOR THE SPECIAL INSPECTOR UNTIL THE REQUIRED INSPECTIONS OR TESTS HAVE BEEN COMPLETED.

SI.9 ANY DEVIATIONS FOUND DURING THE SPECIAL INSPECTION PROCESS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE DESIGNER OF RECORD. ALL DEVIATIONS MUST BE ADDRESSED PRIOR TO COMPLETION OF THE WORK.

SI.10 INSPECTION FREQUENCY:

A. CONTINUOUS – SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.

B. PERIODIC – SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED.

C. OBSERVE – OBSERVE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.

D. PERFORM – PERFORM TASKS FOR EACH JOINT, MEMBER, AND CONNECTION.

SI.11 SPECIAL INSPECTIONS FOR STRUCTURAL, LOAD-BEARING, OR LATERAL LOAD BEARING FABRICATED ITEMS SHALL BE PERFORMED FOR THE FABRICATED ITEMS AT THE FABRICATOR'S SHOP. SPECIAL INSPECTIONS FOR FABRICATED ITEMS MAY BE WAIVED WHEN THE FABRICATOR IS REGISTERED AND HAS APPROVAL TO PERFORM THE WORK WITHOUT SPECIAL INSPECTIONS. IF THE INSPECTIONS ARE WAIVED, THE FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE TO THE DESIGNER OF RECORD SHOWING COMPLIANCE WITH THE APPROVED STRUCTURAL DRAWINGS.

SI.12 SPECIAL INSPECTION REPORTS AND A FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.

Wood Framing

NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	INSPECT PREFABRICATED WOOD STRUCTURAL ELEMENTS AND SITE-BUILT ASSEMBLIES	PERIODIC	IBC 1704.2.5, 1705.5	ATA
2.00	INSPECT GRADING OF WOOD MATERIALS:			
2.01	SAWN LUMBER FRAMING	PERIODIC		ATA
2.02	STRUCTURAL COMPOSITE LUMBER	PERIODIC		ATA
2.03	GLULAM FRAMING	PERIODIC		ATA
3.00	INSPECT FRAMING AND DETAILS:			
3.01	FRAMING LAYOUT, MEMBER SIZES, AND BEARING LENGTHS	PERIODIC		ATA
3.02	BLOCKING AND BRIDGING	PERIODIC		ATA
3.03	HOLES AND NOTCHES (a)	PERIODIC		ATA
4.00	INSPECT CONNECTIONS:			
4.01	BOLTED AND SCREWED CONNECTIONS, INCLUDING DIAMETER, LENGTH, SPACING AND EDGE DISTANCE.	PERIODIC		ATA
4.02	NAILED CONNECTIONS, INCLUDING DIAMETER, LENGTH, TYPE AND SPACING OF NAILS.	PERIODIC		ATA
4.03	PROPRIETARY HANGERS AND FRAMING ANCHORS, INCLUDING FASTENER SIZES AND QUANTITIES.	PERIODIC		ATA
4.04	TIE-DOWN ANCHORS, INCLUDING ANCHOR ROD SIZE AND FASTENER SIZES AND QUANTITIES.	PERIODIC		ATA
5.00	INSPECT SHEAR WALLS AND DIAPHRAGMS:			
5.01	WOOD STRUCTURAL PANEL GRADE AND THICKNESS	PERIODIC		ATA
5.02	FASTENER SIZE, LENGTH AND SPACING	PERIODIC		ATA
5.03	FRAMING MEMBER SIZES AT PANEL EDGES	PERIODIC		ATA
5.04	BLOCKING AT PANEL EDGES	PERIODIC		ATA
5.05	FIELD GLUING	CONTINUOUS		ATA
5.06	HIGH-LOAD DIAPHRAGMS	PERIODIC	IBC 1705.5.1	ATA
6.00	INSPECT METAL-PLATE CONNECTED WOOD TRUSSES:			
6.01	TEMPORARY INSTALLATION RESTRAINT/BRACING AND PERMANENT INDIVIDUAL MEMBER RESTRAINT/BRACING FOR TRUSS SPANNING 60' OR MORE.	PERIODIC	IBC 1705.5.2	ATA
6.02	MULTI-PLY TRUSS CONNECTIONS	PERIODIC		ATA
NOTES:	(a) INSPECTIONS OF HOLES AND NOTCHES TO BE PERFORMED AFTER ELECTRICAL, MECHANICAL AND PLUMBING.			

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Soils

NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC		GEOR
2.00	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC		GEOR
3.00	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	PERIODIC		GEOR
4.00	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT, AND COMPACTION OF COMPACTED FILL.	CONTINUOUS		GEOR
5.00	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC		GEOR

Concrete

NO.	INSPECTION TASK	FREQUENCY	REFERENCE STANDARD	AGENT
1.00	INSPECT REINFORCEMENT AND VERIFY PLACEMENT.	PERIODIC	ACI 318 CH 20, 25.2, 25.3, 26.5.1-26.5.3	ATA
2.00	INSPECT ANCHORS CAST IN CONCRETE.	PERIODIC	ACI 318: 17.8.2	ATA
3.00	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.			ATA
3.01	INSPECT ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	CONTINUOUS	ACI 318: 17.8.2.4	ATA
3.02	INSPECT MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.01.	PERIODIC	ACI 318: 17.8.2	ATA
4.00	VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	ACI 318: CH 19, 26.4.3, 26.4.4; IBC 1904.1, 1904.2, 1908.2, 1908.3	ATA
5.00	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. DETERMINE UNIT WEIGHT OF LIGHTWEIGHT CONCRETE.	CONTINUOUS	ASTM C 172; ASTM C 31; ACI 318:26.4.5, 26.12; IBC 1908.10	ATA
6.00	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	ACI 318: 26.4.7-26.4.9; IBC 1908.9	ATA
6.01	VERIFY THAT CONCRETE SPECIMENS FOR STRENGTH TESTS ARE MAINTAINED IN THE REQUIRED INITIAL CURING AND LABORATORY CURING ENVIRONMENT, AND THAT THE MAXIMUM AND MINIMUM TEMPERATURES DURING THE INITIAL CURING PERIOD ARE REPORTED.	CONTINUOUS	ACI 318: 26.12; ASTM C 31	
7.00	INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	PERIODIC	ACI 318: 26.10.1(B)	ATA
8.00	ISOLATED CONCRETE FOOTINGS OF BUILDINGS THREE STORIES OR LESS ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (1)	ATA
9.00	CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF LIGHT-FRAME CONSTRUCTION OR THOSE THAT ARE DESIGNED IN ACCORDANCE WITH IBC 2015 TABLE 1809.7 ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (2)	ATA
10.00	SLABS ON GRADE ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (3)	ATA
11.00	CONCRETE FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH IBC 2015 TABLE 1807.1.6.2 ARE EXCEPTED FROM INSPECTIONS BUT NOT FROM MATERIALS TESTING.		IBC 1705.3 (4)	ATA

Cold-Formed Framing

NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	PRIOR TO ASSEMBLY OR INSTALLATION:		AISI S240 TABLE D6.5-1	
1.01	VERIFY COMPLIANCE OF STRUCTURAL MEMBERS FOR PRODUCT IDENTIFICATION	PERFORM	AISI S240 SECTION A5.5	ATA
1.02	VERIFY COMPLIANCE OF CONNECTORS	PERFORM		ATA
1.03	DOCUMENT ACCEPTANCE OR REJECTION OF COLD-FORMED STEEL STRUCTURAL MEMBERS AND CONNECTORS	PERFORM		ATA
2.00	AFTER ASSEMBLY OR INSTALLATION:		AISI S240 TABLE D6.5-2	
2.01	VERIFY COMPLIANCE OF COLD-FORMED STEEL STRUCTURAL MEMBERS FOR PRODUCT IDENTIFICATION	PERFORM	AISI S240 SECTION A5.5	ATA
2.02	VERIFY COMPLIANCE OF CONNECTORS	PERFORM		ATA
2.03	DOCUMENT ACCEPTANCE OR REJECTION OF COLD-FORMED STEEL STRUCTURAL MEMBERS AND CONNECTORS	PERFORM		ATA
3.00	PRIOR TO MECHANICAL FASTENING:		AISI S240 TABLE D6.7-1	
3.01	MECHANICAL FASTENER MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS	OBSERVE		ATA
3.02	PROPER TOOLS AVAILABLE FOR MECHANICAL FASTENER INSTALLATION	OBSERVE		ATA
3.03	PROPER STORAGE FOR MECHANICAL FASTENERS	OBSERVE		ATA
4.00	DURING MECHANICAL FASTENING:		AISI S240 TABLE D6.7-2	
4.01	MECHANICAL FASTENERS ARE POSITIONED AS REQUIRED	OBSERVE		ATA
4.02	MECHANICAL FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS	OBSERVE		ATA
5.00	AFTER MECHANICAL FASTENING:		AISI S240 TABLE D6.7-3	
5.01	VERIFY COMPLIANCE OF MECHANICAL FASTENERS	PERFORM		ATA
5.02	REPAIR ACTIVITIES	PERFORM		ATA
5.03	DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICALLY FASTENED CONNECTIONS	PERFORM		ATA
6.00	AFTER INSTALLATION OF COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION:		AISI S240 TABLE D6.8-1	
6.01	VERIFY COMPLIANCE OF COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION	PERFORM		ATA
6.02	DOCUMENT ACCEPTANCE OR REJECTION OF COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION	PERFORM		ATA

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PANDA PROJECT #: D24956
PANDA STORE #: TBD
ARCH PROJECT #: 239201

ARCHITECT LOGO

bdg architects

PANDA EXPRESS

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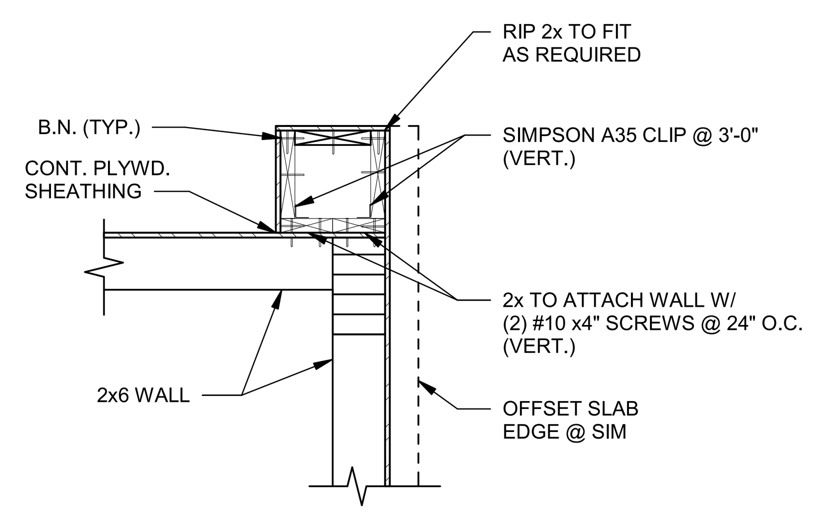
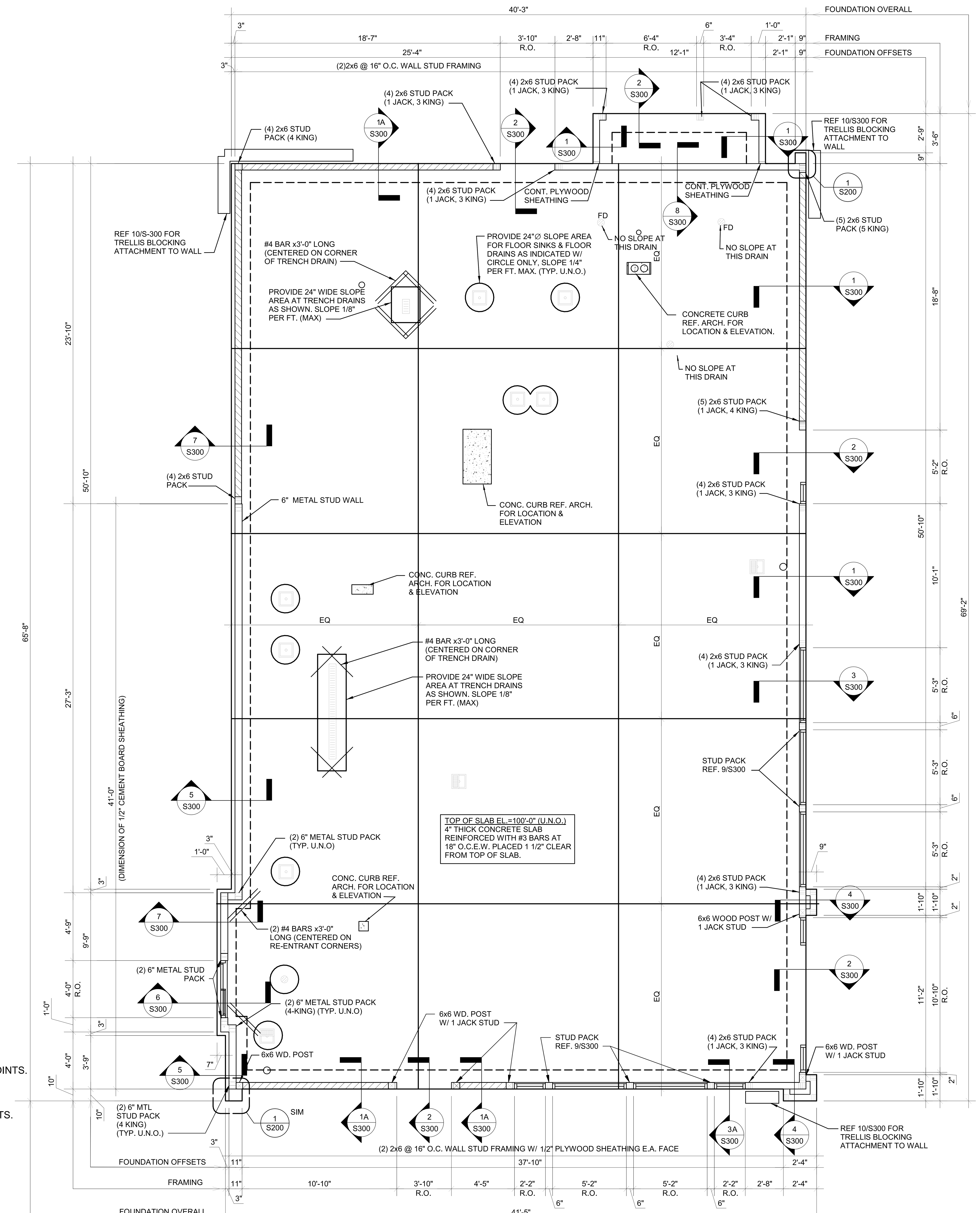
SPECIAL INSPECTIONS

S106



1 Detail
S200 3/4" = 1'-0"



PROJECT NORTH
TRUE NORTH

Foundation and Floor Plan

1/4" = 1'-0"

1. FOR SLAB ON GRADE CONSTRUCTION, SEE GENERAL NOTES AND TYPICAL DETAILS. FOR SLAB EDGES NOT DIMENSIONED, SEE ARCHITECTURAL DRAWINGS.
2. COORDINATE WITH ARCHITECTURAL, MECHANICAL AND PLUMBING DRAWINGS FOR LOCATIONS OF ALL DEPRESSIONS, CURBS, DRAINS, ETC. IN FLOOR SLAB.
3. GENERAL CONTRACTOR SHALL COORDINATE TILE JOINT LOCATIONS WITH CONTROL JOINTS.
4. FOOTING STEP ELEVATIONS AND LOCATION ARE APPROXIMATE. GENERAL CONTRACTOR SHALL COORDINATE ALL FOOTING STEPS WITH CIVIL, PLUMBING AND UTILITY DRAWINGS.
5. PROVIDE 2#5x6'-0" AT ALL RE-ENTRANT CORNERS AND DISCONTINUOUS CONTROL JOINTS.
6. ▨ INDICATES WOOD SHEAR WALL - SEE GENERAL NOTES. PROVIDE HOLD-DOWNS EACH END - SEE TYPICAL DETAILS.

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PANDA EXPRESS CHINESE KITCHEN

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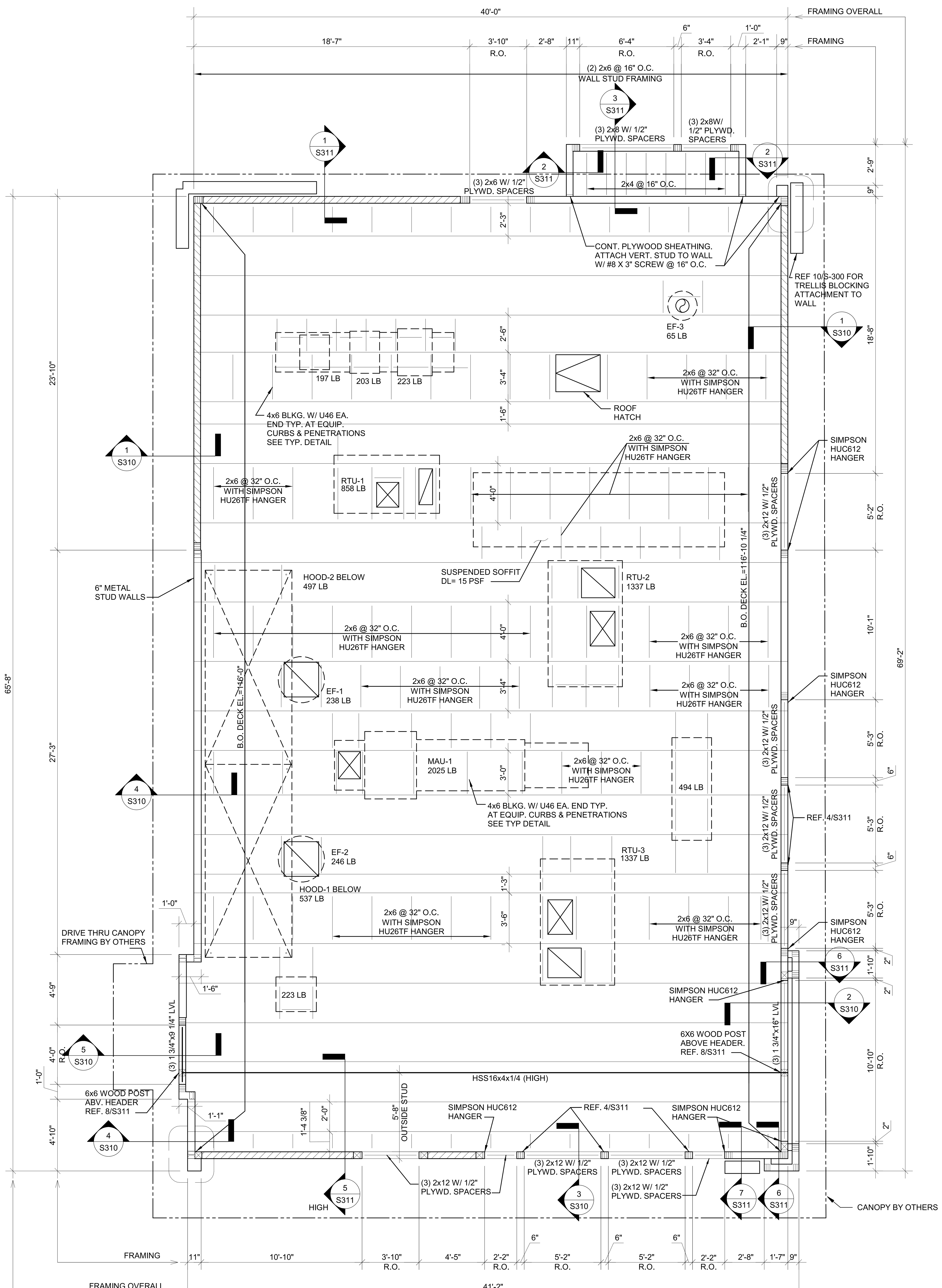
FOUNDATION AND FLOOR PLAN

S200



PROJECT NORTH

TRUE NORTH

Roof Framing Plan

1/4" = 1'-0"

1. TRUSS BEARING ELEVATION: 113'-6".
2. ROOF SYSTEM: BUILDING: 3/4" PLYWOOD SHEATHING ON WOOD TRUSSES (AT 2'-8" TYPICAL) ON LOAD-BEARING WOOD STUDS, UNLESS NOTED OTHERWISE. SEE GENERAL NOTES.
3. LONG DIMENSION OF PLYWOOD TO BE PERPENDICULAR TO ROOF TRUSSES.
4. TRUSS LAYOUT SHOWN FOR GENERAL INTENT. EXACT LAYOUT OF WOOD TRUSSES TO BE DETERMINED BY THE TRUSS MANUFACTURER AND SUBMITTED TO THE STRUCTURAL ENGINEER.
5. CONTRACTOR COORDINATE SIZE, LOCATION, AND WEIGHT OF ANY MECHANICAL, ELECTRICAL, PLUMBING, OR MISCELLANEOUS LOADS SUPPORTED BY THE WOOD TRUSSES BY EQUIPMENT SUPPLIERS.
6. TEMPORARY AND PERMANENT LATERAL BRACING MUST BE DESIGNED BY TRUSS MANUFACTURER.
7. SEE GENERAL NOTES AND TYPICAL DETAILS FOR DEAD LOAD, LIVE LOAD, SNOW LOAD AND WIND LOAD ON TRUSSES.
8. ALL NEW ROOFTOP EQUIPMENT TO BE CURB MOUNTED AND TIED DOWN PER MANUFACTURER'S INSTALLATION REQUIREMENTS. SEE TYPICAL DETAILS FOR ADDITIONAL INFORMATION.
9. INDICATES WOOD SHEAR WALL - SEE GENERAL NOTES. PROVIDE HOLD-DOWNS EACH END - SEE TYPICAL DETAILS.

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ROOF FRAMING PLAN

S201

PANDA HOME 2600 R3

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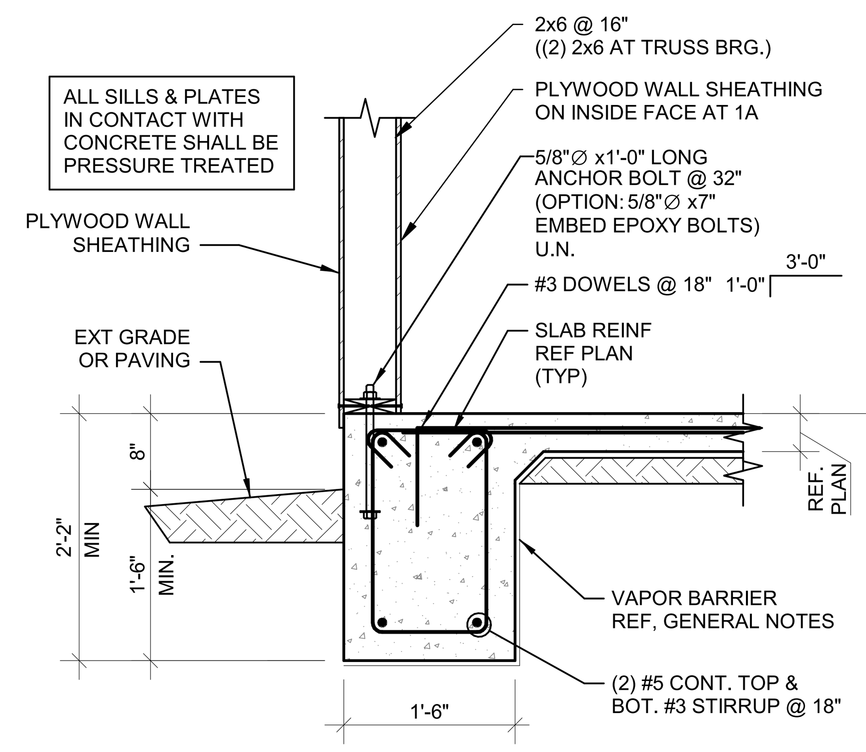
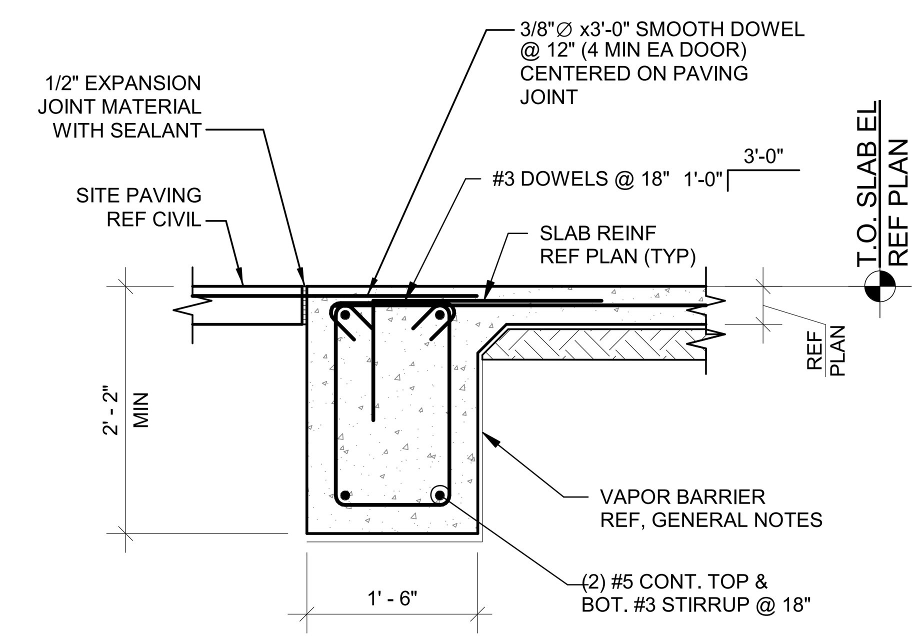
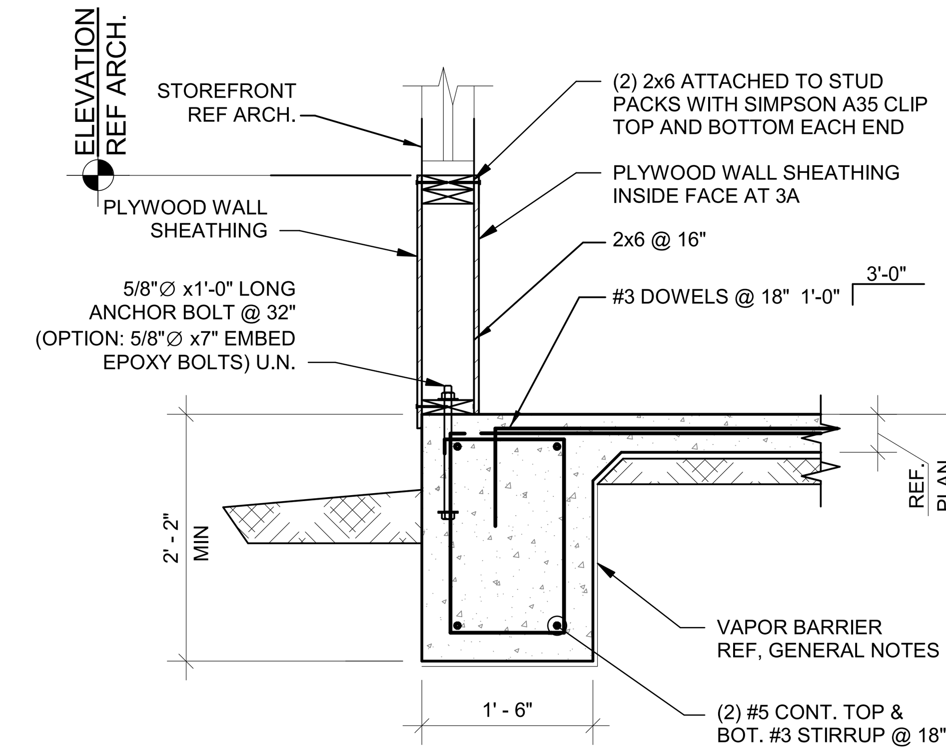


1A/1 Section
S300 3/4" = 1'-0"



2 Section
S300 3/4" = 1'-0"



3A/3 Section
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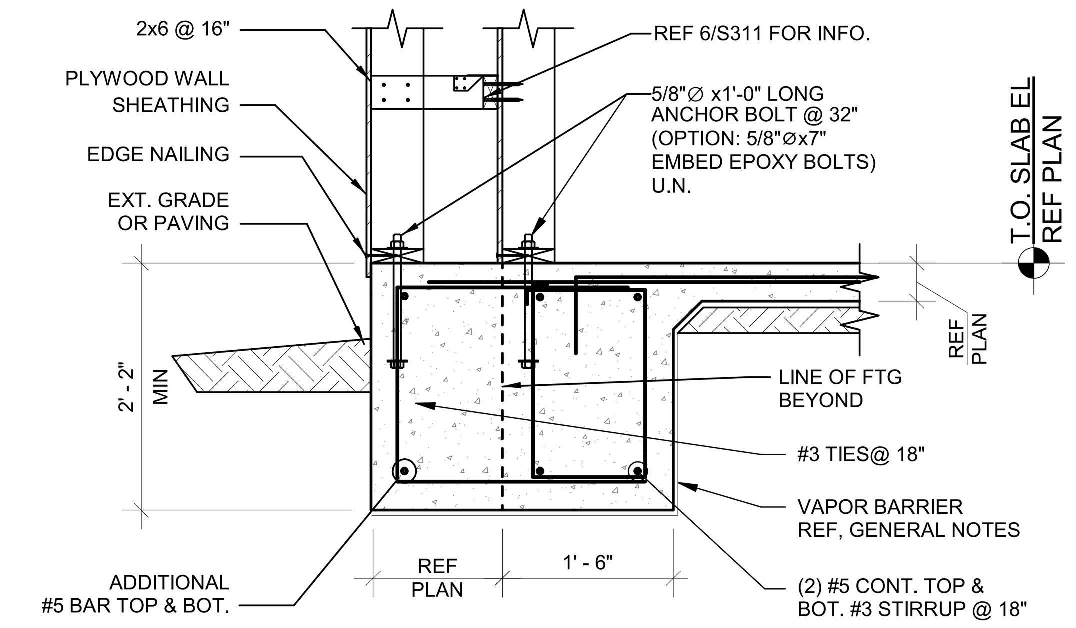
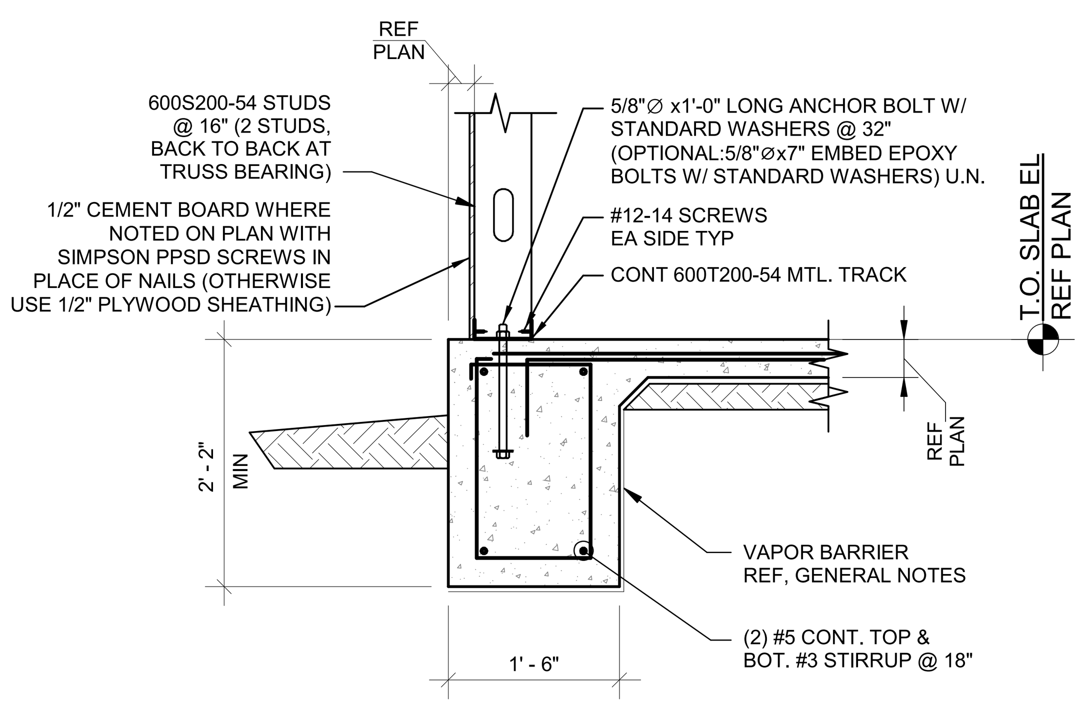
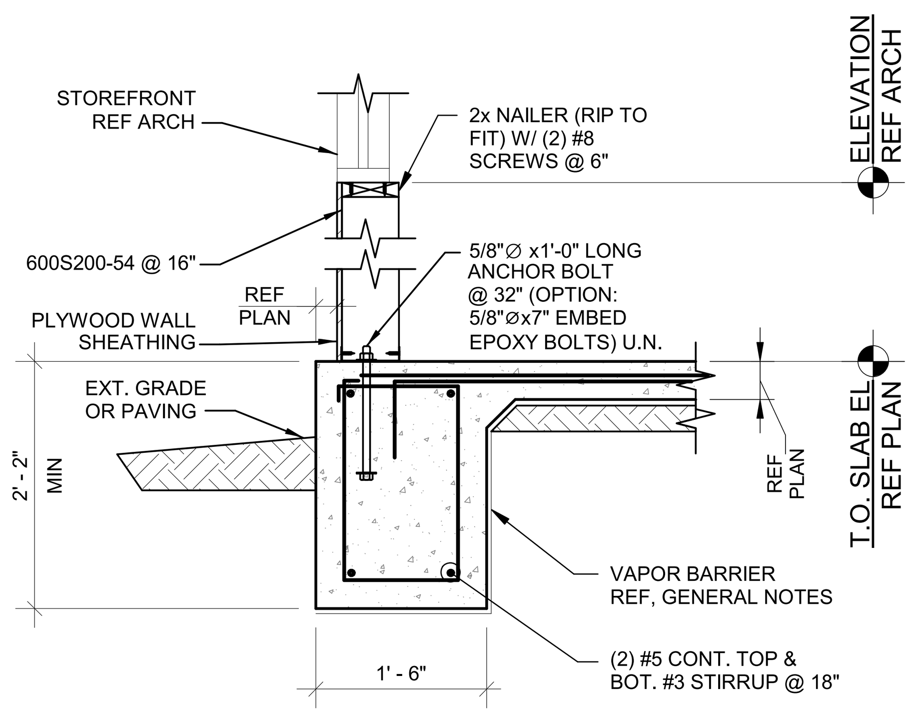


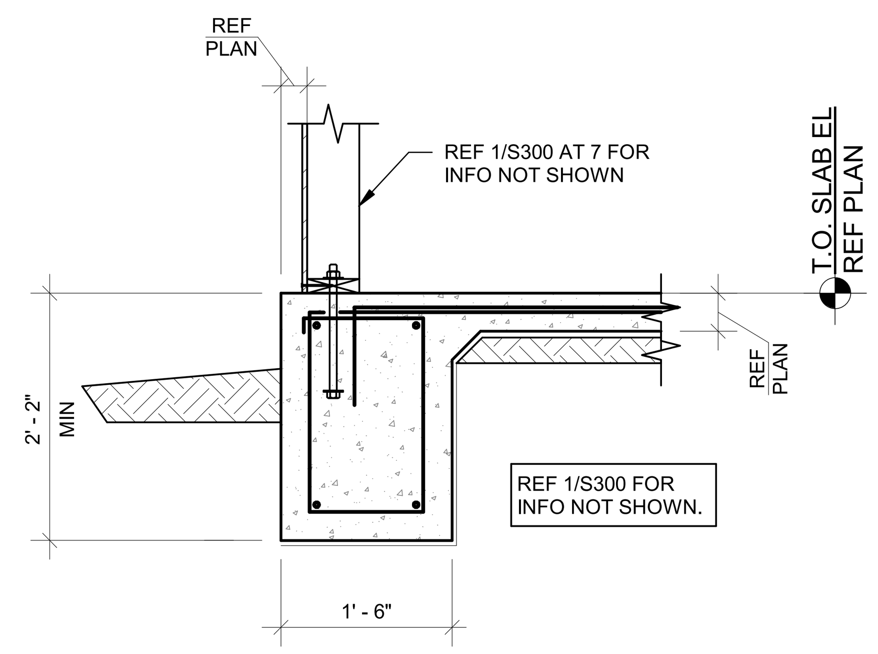
4 Section
S300 3/4" = 1'-0"



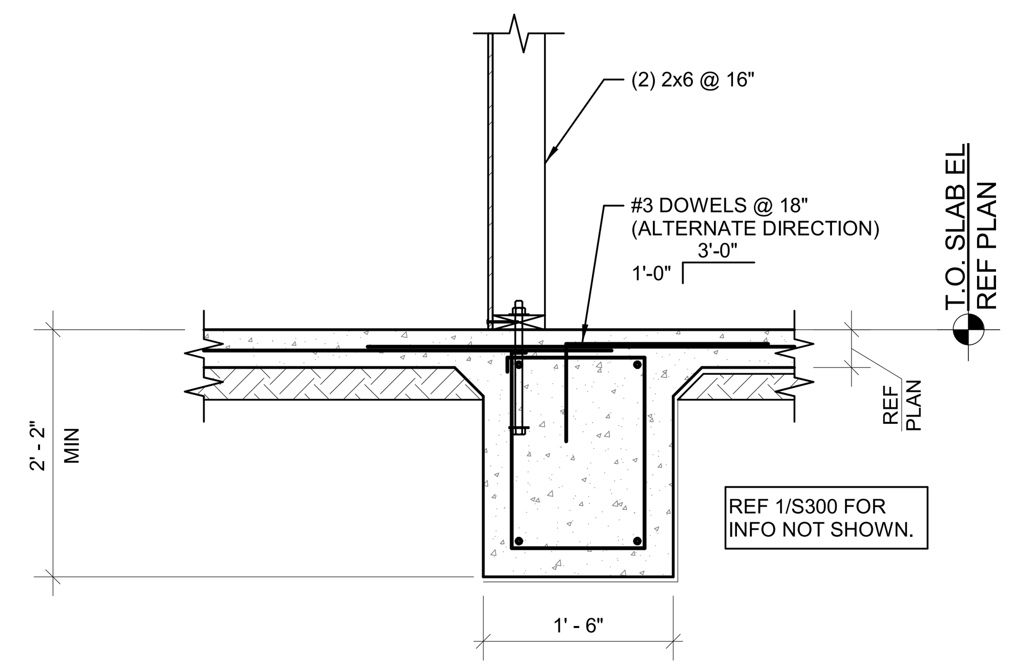
5 Section
S300 3/4" = 1'-0"



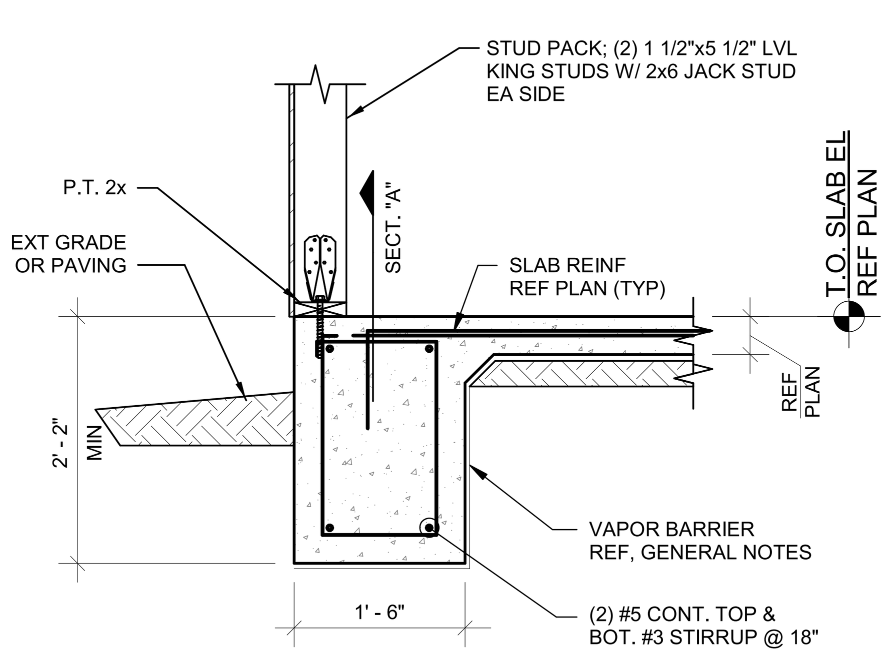
6 Section
S300 3/4" = 1'-0"



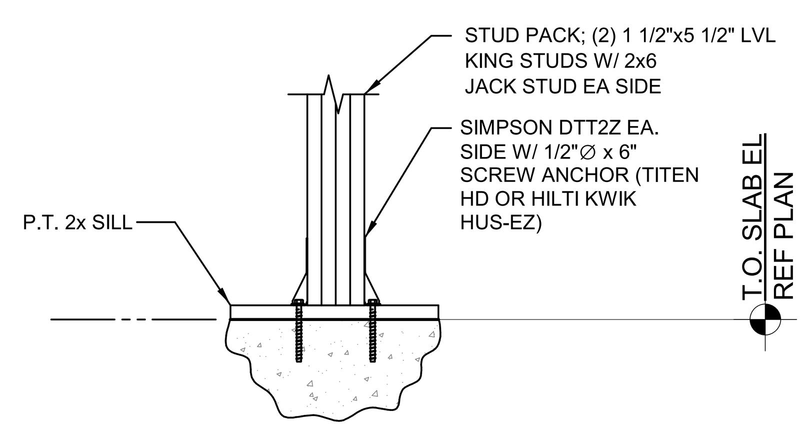
7 Section
S300 3/4" = 1'-0"



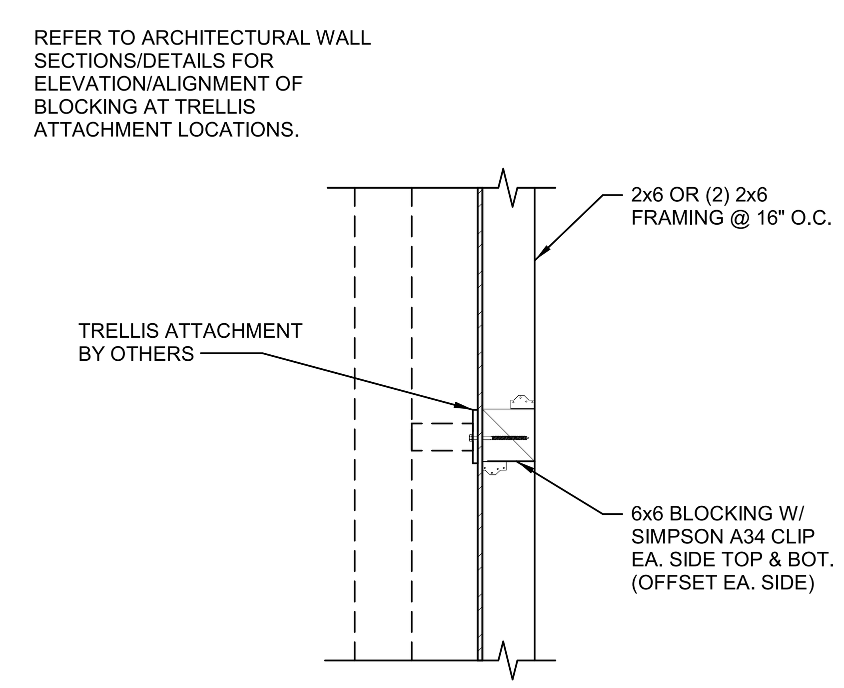
8 Section
S300 3/4" = 1'-0"



9 Section
S300 3/4" = 1'-0"



SECT. "A"



10 Section
S300 3/4" = 1'-0"

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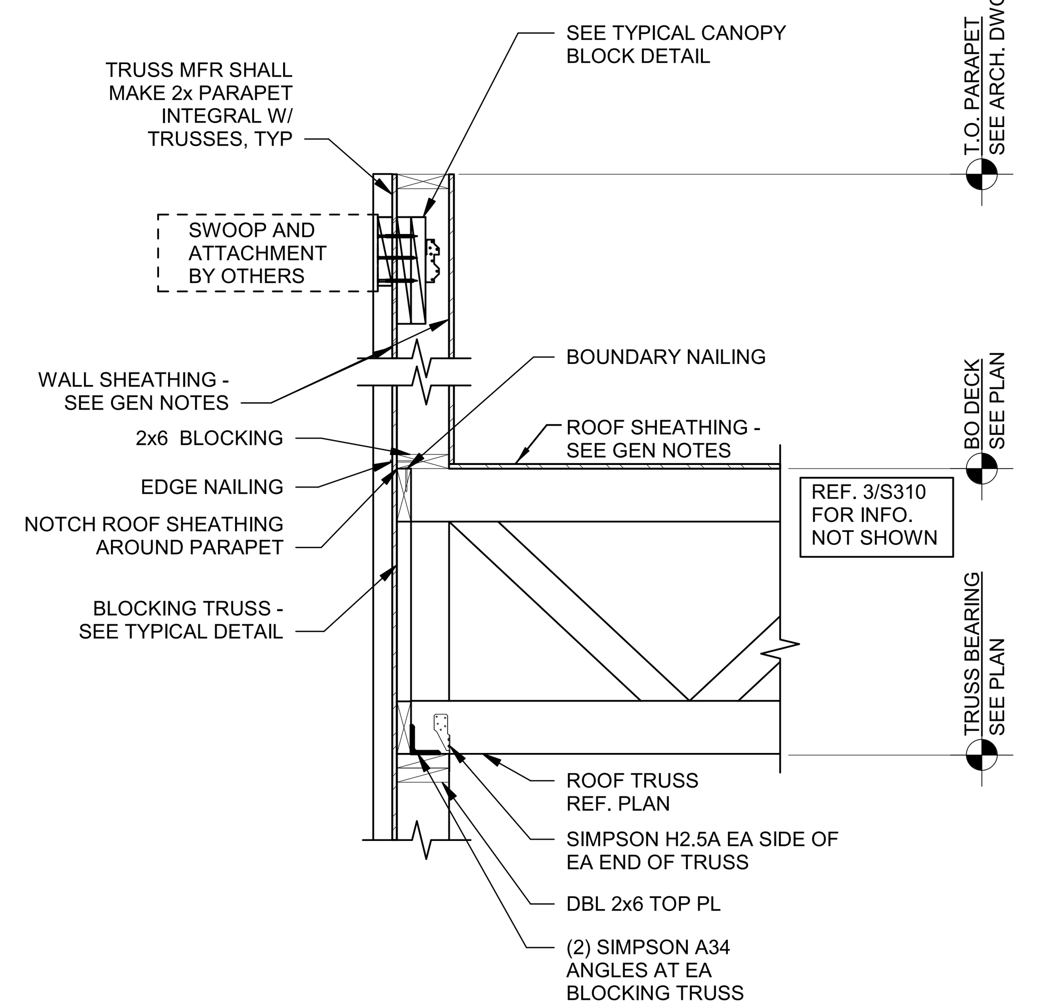
FOUNDATION SECTIONS

S300



1A/1 Section
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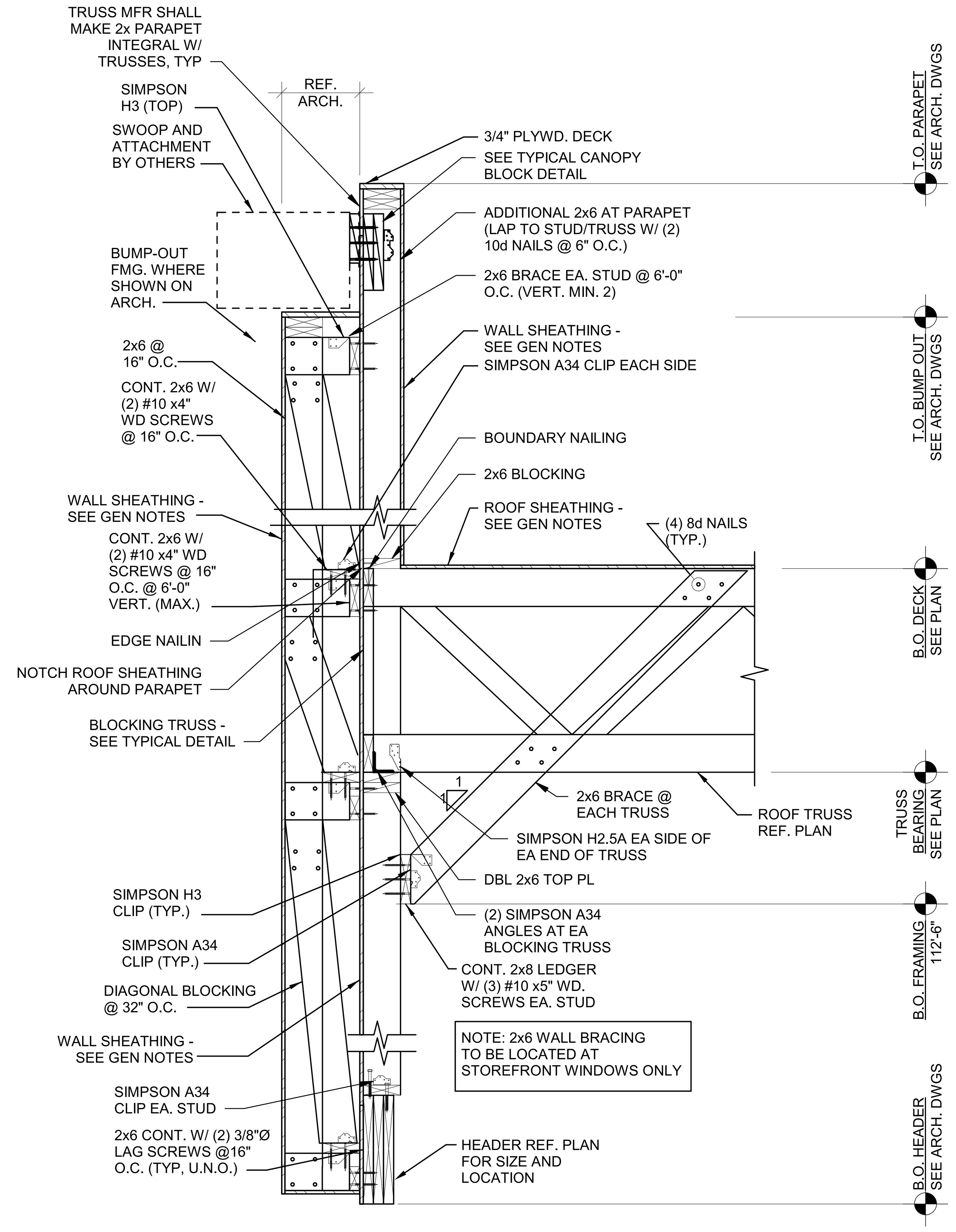

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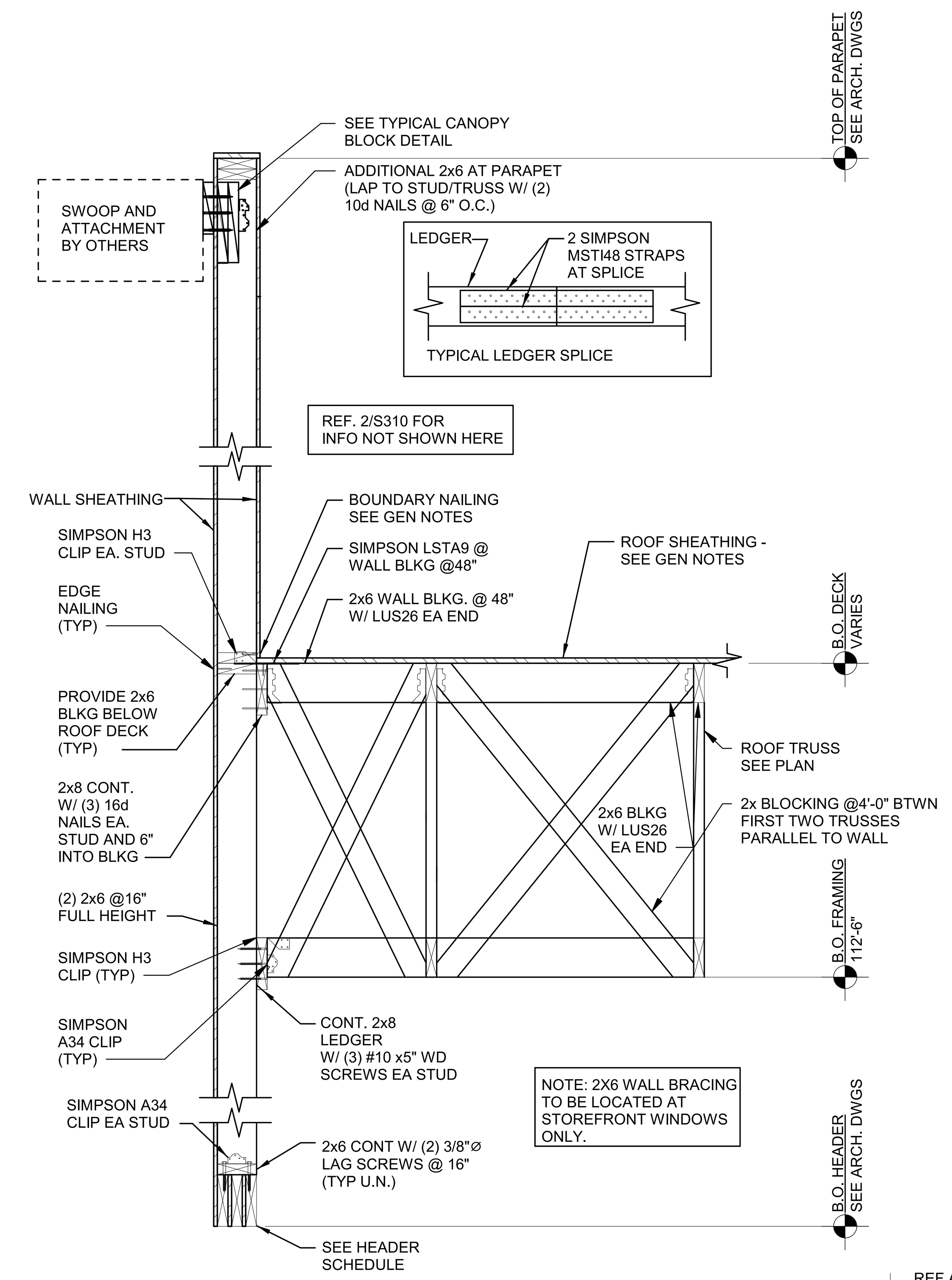

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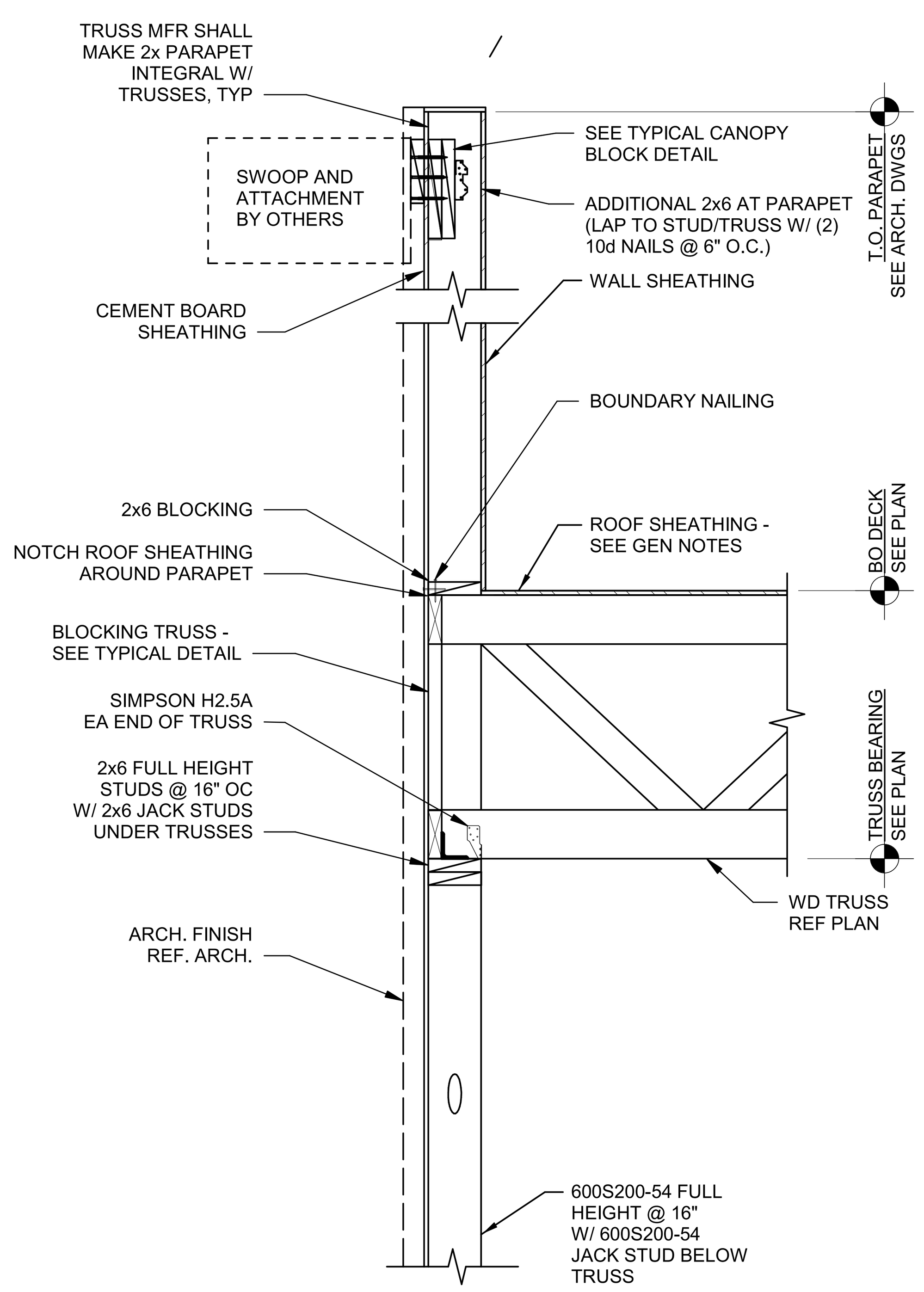

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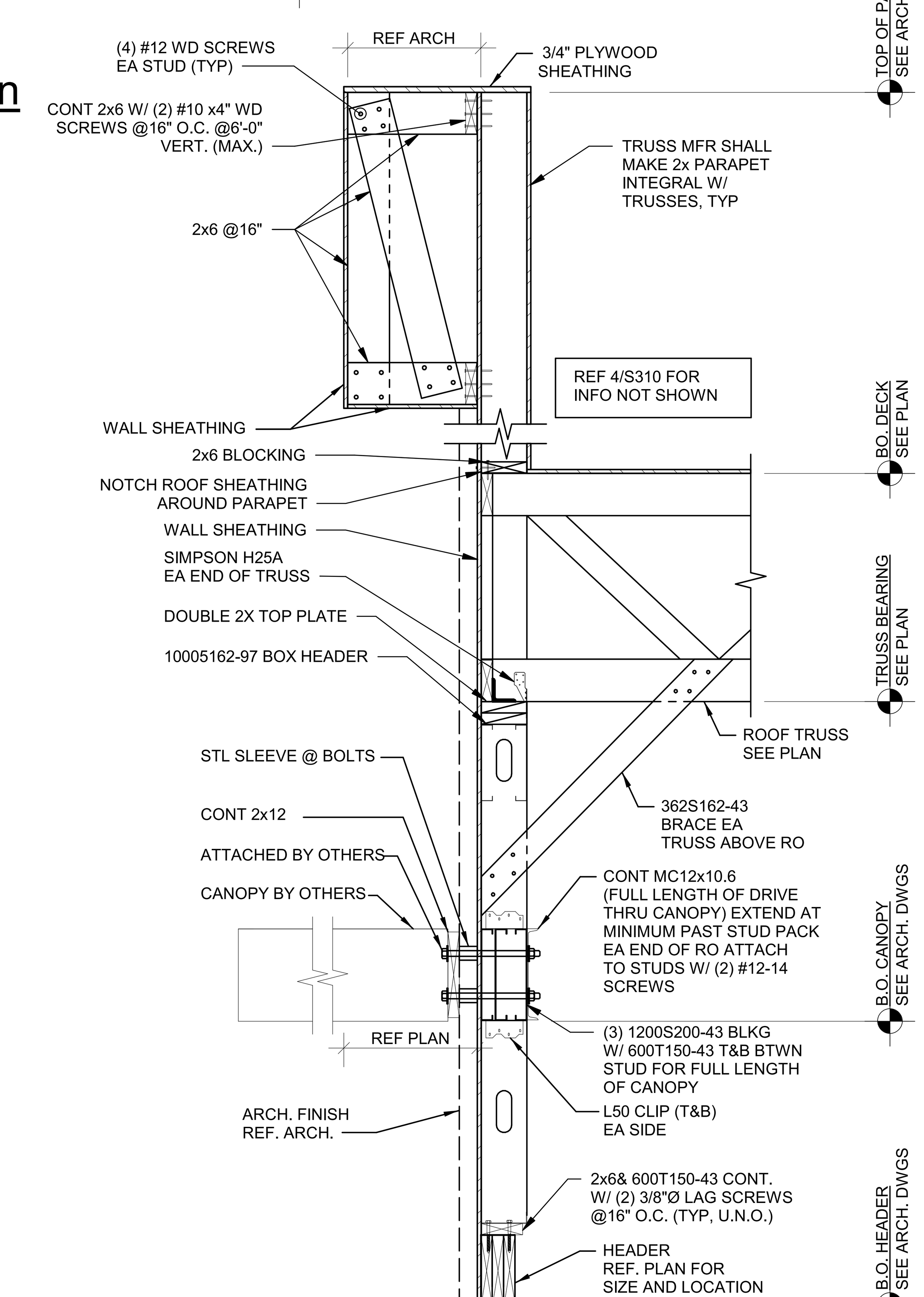

5 Section
S310 3/4" = 1'-0"

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Phone (205) 251-4500
Project No. 131410.005

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ROOF SECTIONS

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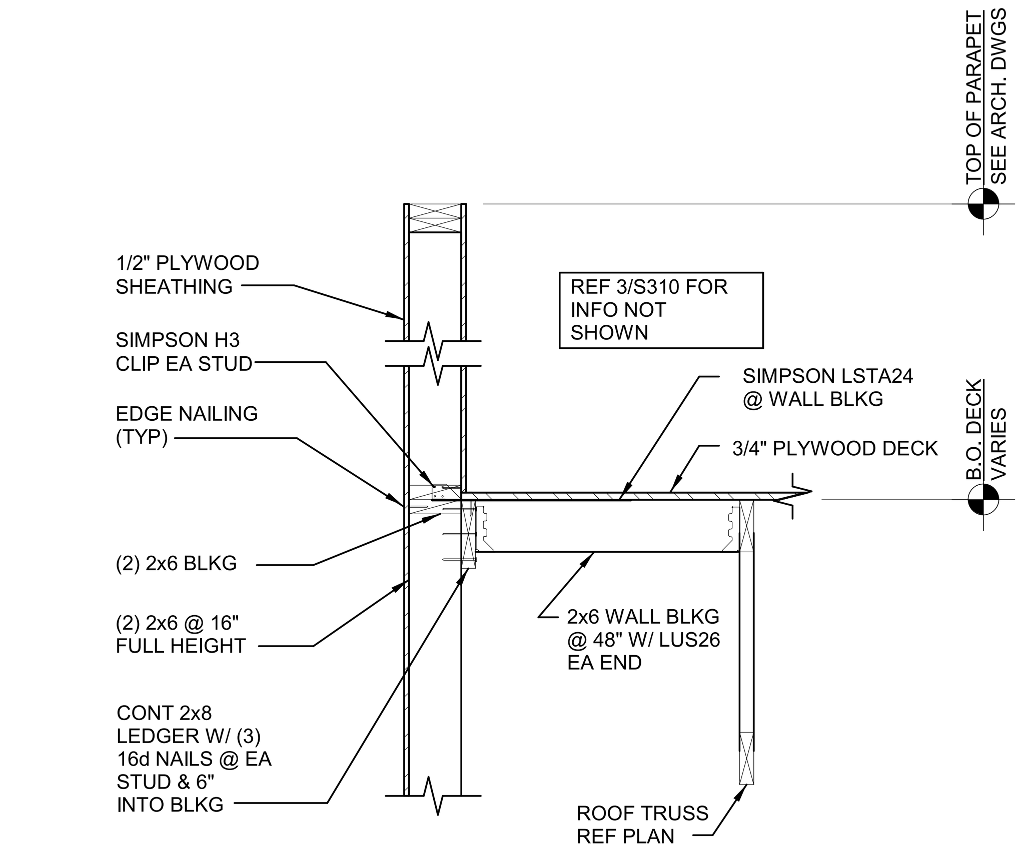

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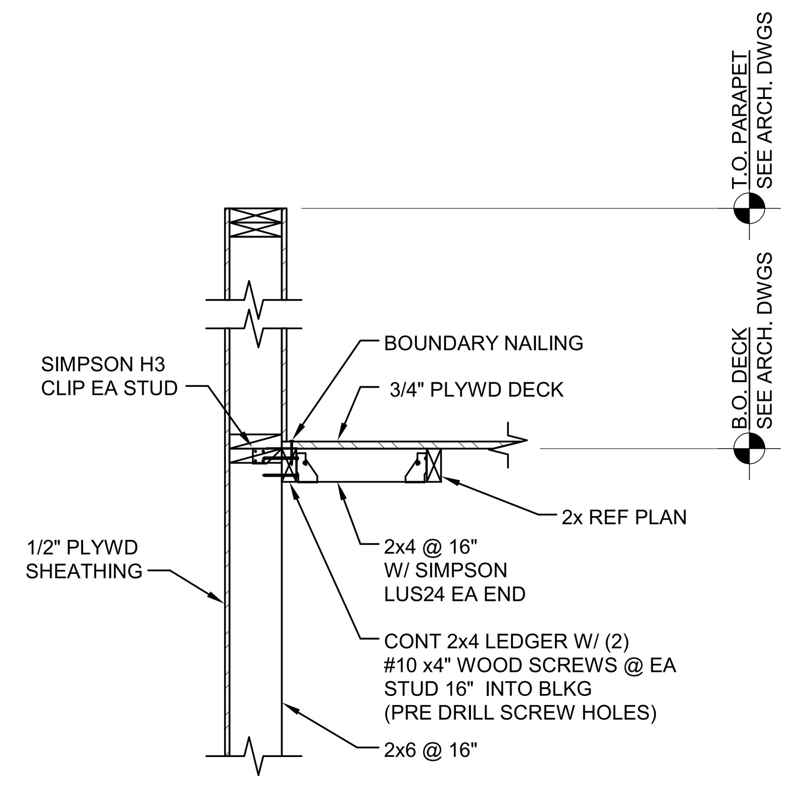

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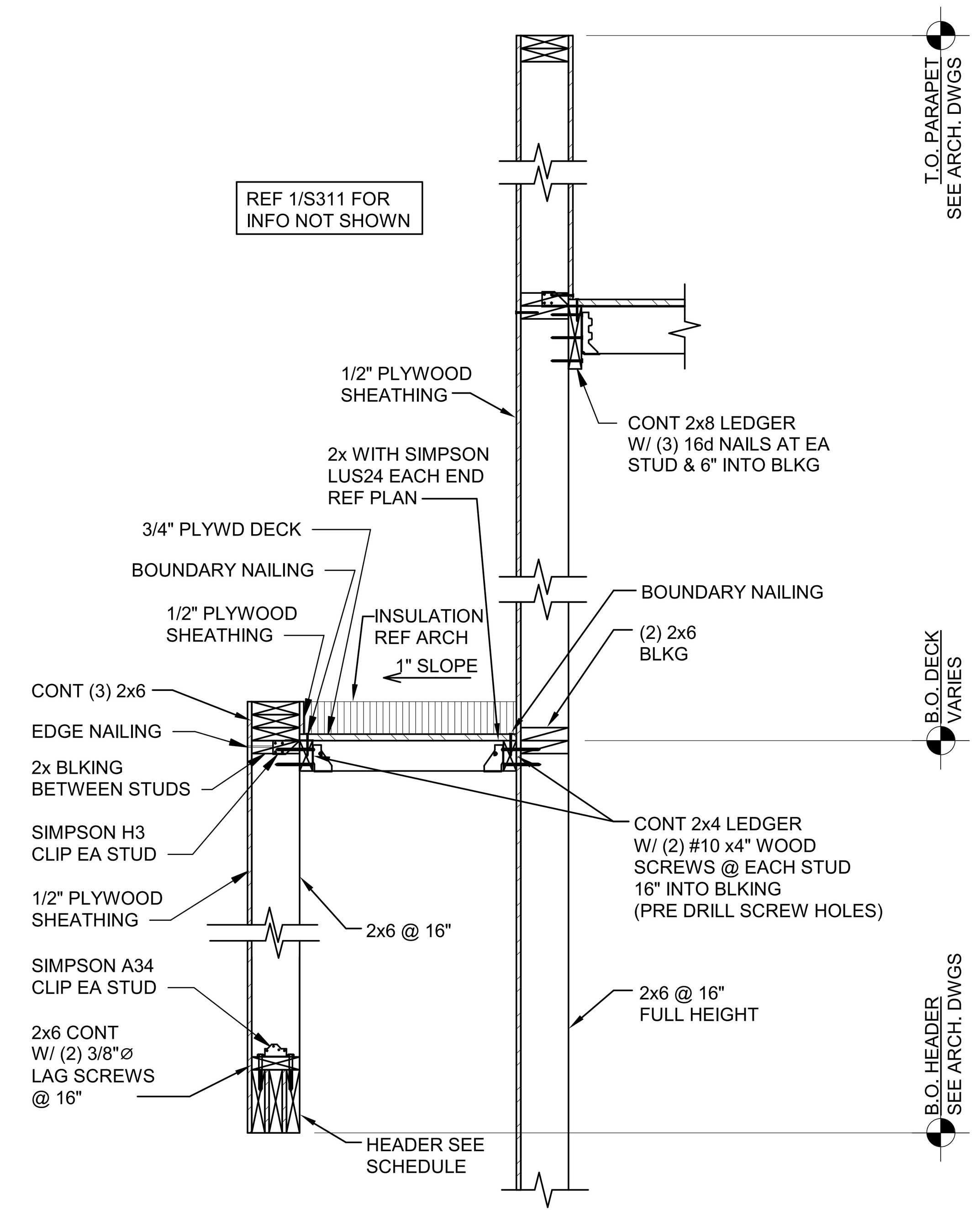

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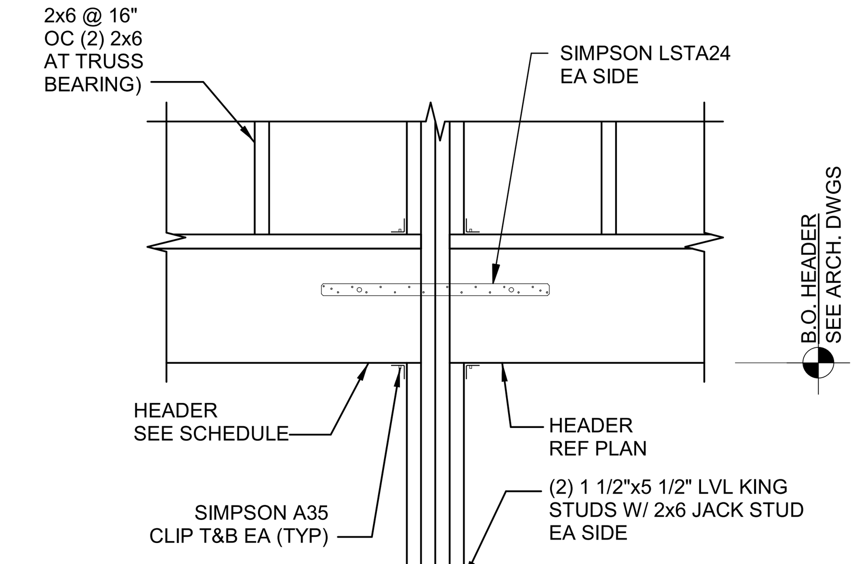

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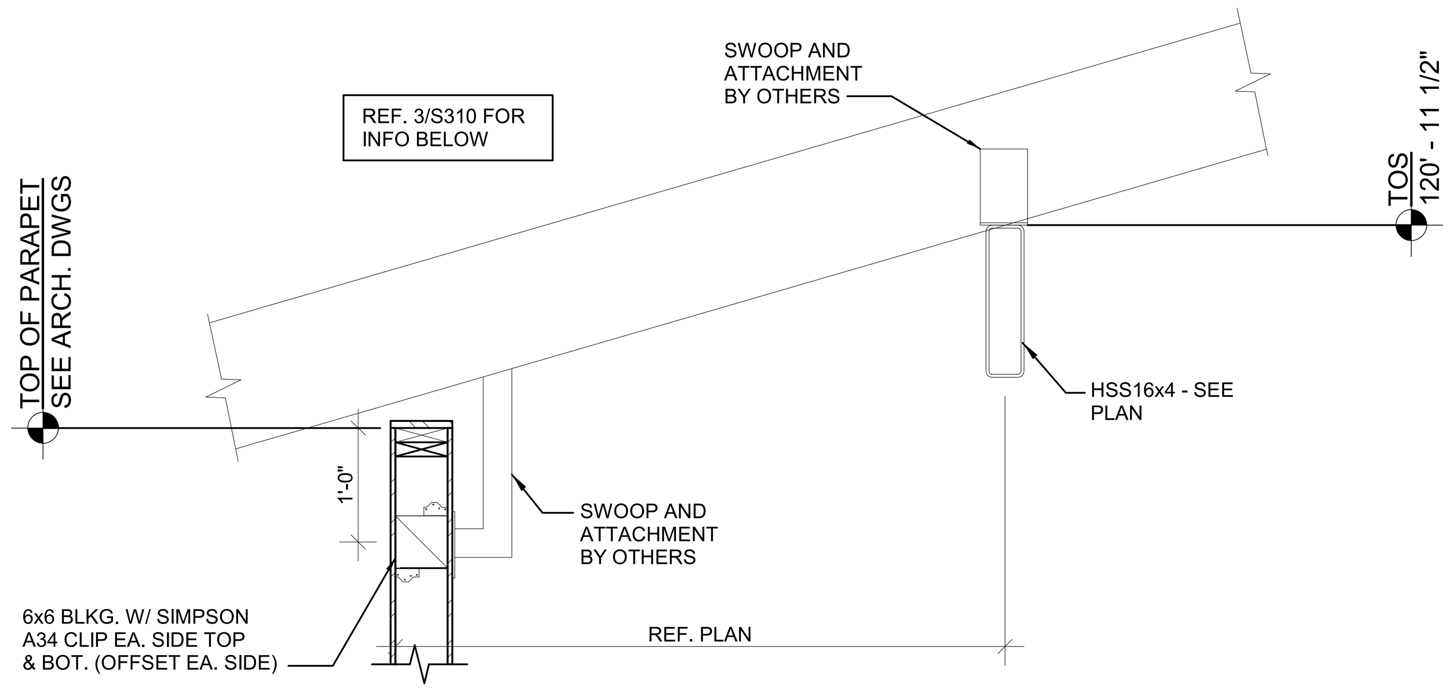

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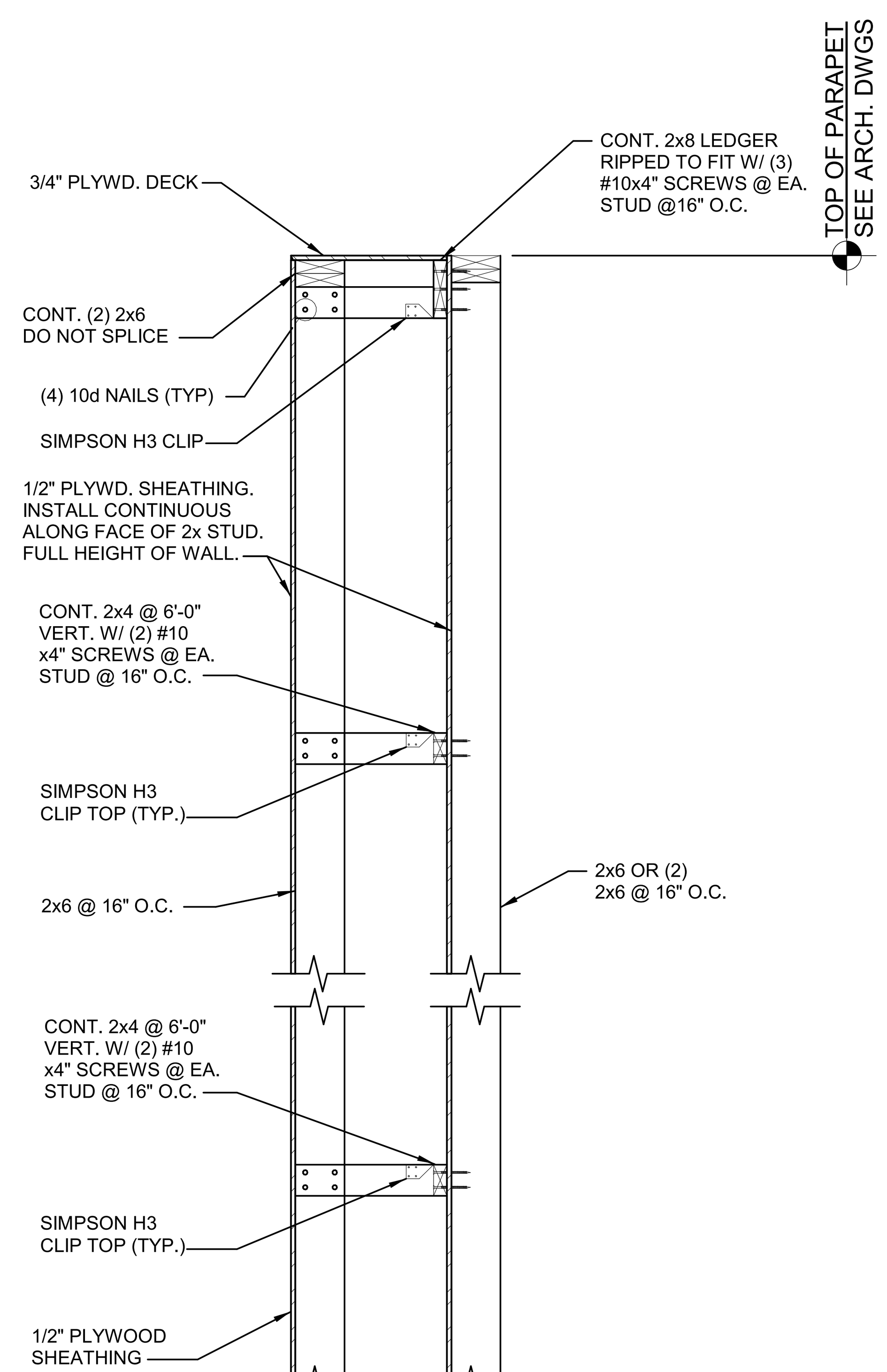

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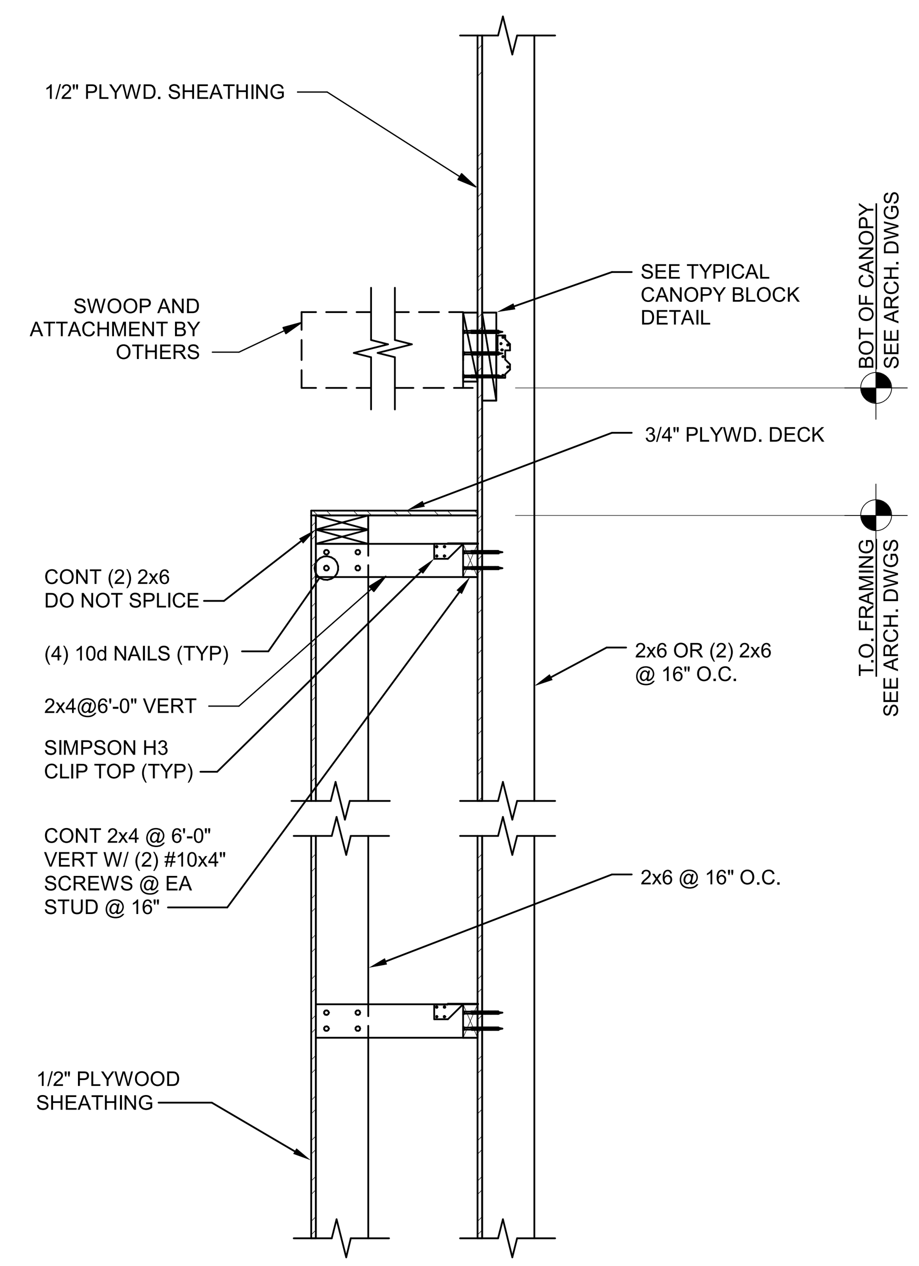

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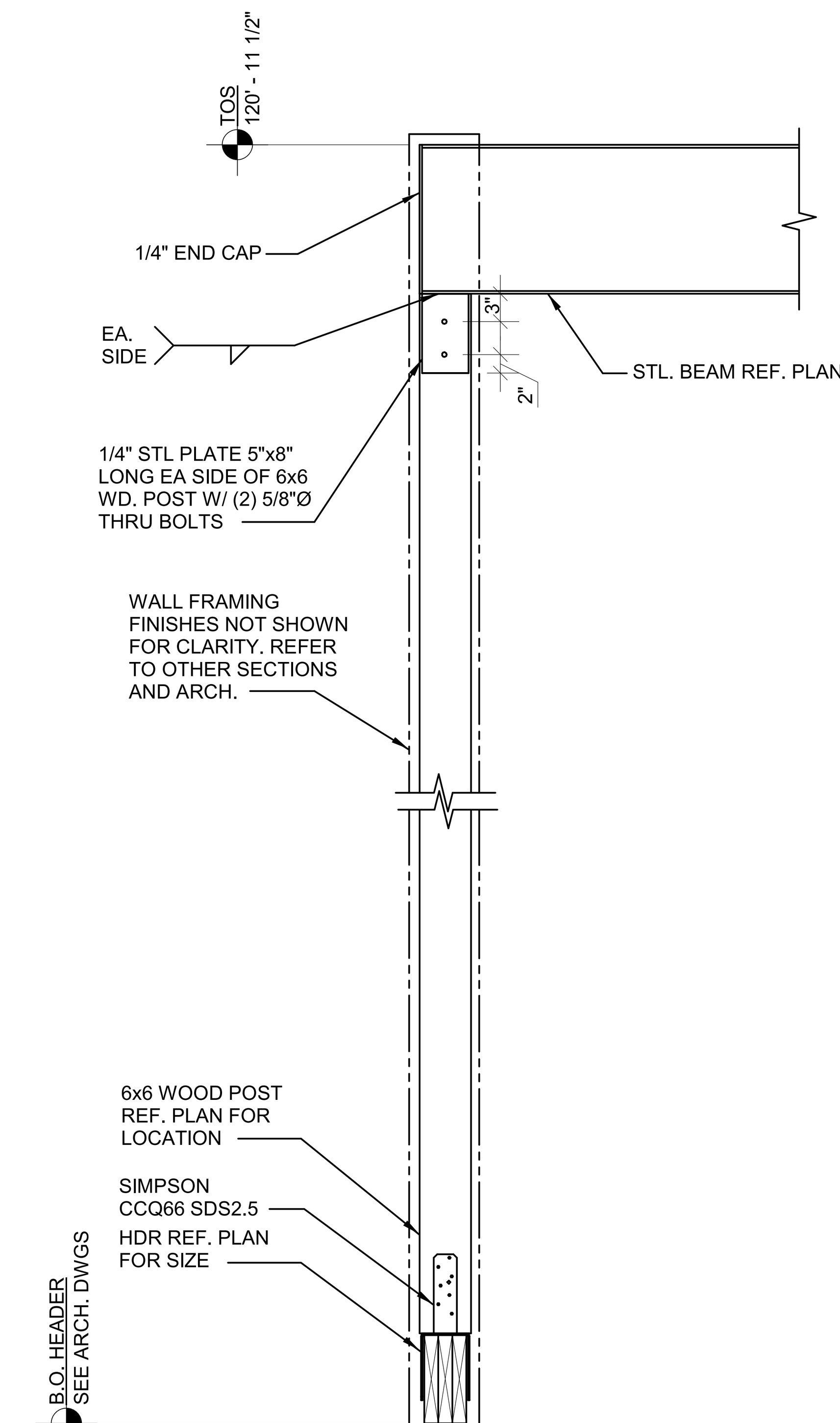

8 Section
S311 3/4" = 1'-0"

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