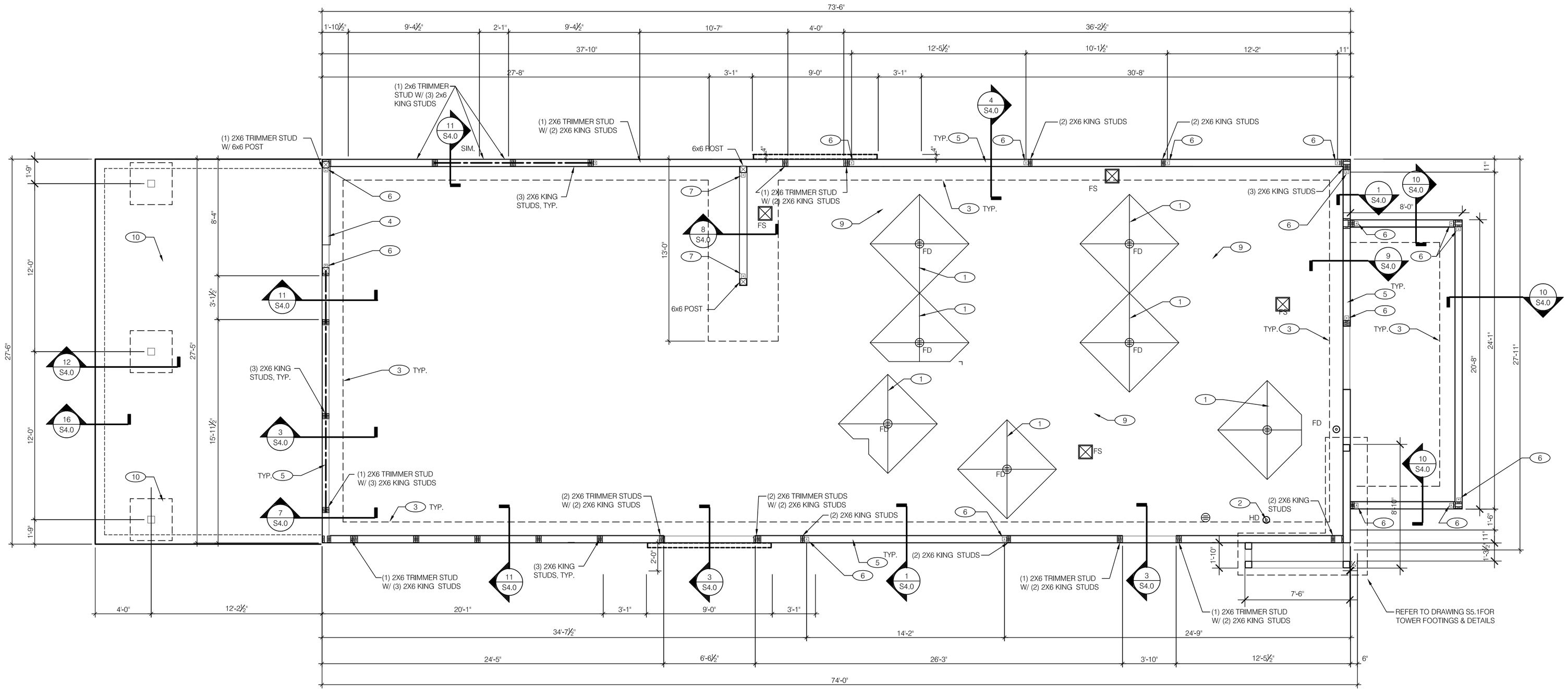

12 | 11 | 10 | 9 | 8 | 7 | 6 | 5 | 4 | 3 | 2 | 1

FOUNDATION PLAN 1/4"=1'-0" **A**

DESIGN CRITERIA:
2012 INTERNATIONAL BUILDING CODE

ROOF LOADS:
LIVE LOAD: 20 PSF
DEAD LOAD: 15 PSF

WIND LOADS:
ULTIMATE WIND SPEED: 115 MPH
RISK CATEGORY: II
EXPOSURE CATEGORY: B
INTERNAL PRESSURE COEFF.: ±0.18

SEISMIC LOADS:
SEISMIC IMPORTANCE FACTOR: 1.0
SITE CLASS: D

MAPPED SPECTRAL RESPONSE ACCEL:
Ss: 0.26
S1: 0.090

SPECTRAL RESPONSE COEFF.:
SHORT PERIODS (SDS): 0.242
1 SEC. PERIODS (SD1): 0.144

SEISMIC DESIGN CATEGORY: B

WOOD SHEAR WALLS
RESPONSE MOD. FACTOR (R): 6.5

DESIGN BASE SHEAR (V): 0.037W

ANALYSIS BY SIMPLIFIED PROCEDURE

PROVIDE SHOP DRAWINGS AND CALCULATIONS BY REGISTERED ENGINEER FOR SIGNS AND AWNINGS

DESIGN CRITERIA **F**

FOUNDATION

1. FOUNDATION DESIGN IS BASED UPON THE GEOTECHNICAL ENGINEERING REPORT ENTITLED "REPORT NAME HERE" BY "COMPANY NAME HERE" DATED "DATE HERE", PROJECT NO. ######.
2. CONTRACTOR TO PROVIDE FOUNDATION & FOOTING AS REQUIRED FOR PYLON OR MONUMENTAL SIGN, SEE ELECTRICAL DRAWINGS FOR DETAIL.
3. COORDINATE STRUCTURAL PLANS AND DETAILS WITH REQUIREMENTS OF GEOTECHNICAL REPORT, FOUNDATION DESIGN IS BASED ON 1,500 PSF ALLOWABLE BEARING CAPACITY,
4. CONTRACTOR SHALL TREAT SOIL BELOW SLAB FOR TERMITES,
5. REFER TO THE GEOTECHNICAL REPORT FOR GENERAL REQUIREMENTS OF EARTHWORK, OVEREXCAVATION, SUBGRADE PREPARATION, FILL AND COMPACTION, WATERPROOFING AND OTHER PERTINENT REQUIREMENTS AND INFORMATION,
6. PROTECT PIPES AND CONDUITS RUNNING THROUGH WALLS AND SLABS WITH 1/2 INCH EXPANSION MATERIAL. LOWER CONTINUOUS FOOTINGS AND GRADE BEAMS PERPENDICULAR TO PIPE RUNS TO ALLOW PIPES TO PASS ABOVE THE FOOTINGS OR THROUGH THE GRADE BEAMS. ALTERNATIVELY, PROVIDE A CONCRETE JACKET IF PIPES ARE LOW ENOUGH TO BE PLACED BELOW THE FOOTINGS AND GRADE BEAMS, LOWER FOOTINGS AND GRADE BEAMS PARALLEL TO PIPE RUNS TO AVOID SURCHARGE ONTO ADJACENT TRENCH EXCAVATIONS,
7. MAINTAIN SUBGRADE AND FILL MOISTURE CONTENT UNTIL FOUNDATIONS ARE PLACED,
8. ARRANGE FOR OWNER'S INDEPENDENT TESTING AGENCY TO MONITOR CUT AND FILL OPERATIONS AND PERFORM FIELD DENSITY AND MOISTURE CONTENT TESTS TO VERIFY COMPACTION AND APPROVE FOOTING SUBGRADES PRIOR TO PLACING CONCRETE.
9. DO NOT PLACE FOOTINGS OR SLABS AGAINST SUBGRADE CONTAINING FREE WATER, FROST, OR ICE,
10. MAINTAIN PROPER SITE DRAINAGE DURING CONSTRUCTION TO ENSURE SURFACE RUNOFF AWAY FROM STRUCTURES AND TO PREVENT PONDING OF SURFACE RUNOFF NEAR THE STRUCTURES.

SUBGRADE PAD PREPARATION RECOMMENDATIONS

1. THESE SUMMARY RECOMMENDATIONS ARE PROVIDED FROM THE GEOTECHNICAL INVESTIGATION,
2. UNLESS SPECIFICALLY INDICATED OTHERWISE IN THE DRAWINGS, THE LIMITS OF THIS SUBSURFACE PREPARATION ARE CONSIDERED TO BE THAT PORTION OF THE SITE DIRECTLY BENEATH THE BUILDING AND FIVE FEET BEYOND. BUILDING IS REFERRING TO AND INCLUDES ALL AREAS AS SHOWN ON SHEET S1.0, THE VAPOR RETARDER, WHERE REQUIRED, DOES NOT EXTEND BEYOND THE LIMITS OF THE ACTUAL BUILDING.
3. ALL VEGETATION, CONCRETE, STUMPS, BRUSH, ABANDONED STRUCTURES, ROOTS, RUBBISH, AND ANY OTHER UNDESIRABLE MATTER SHOULD BE REMOVED AND STRIPPED TO A DEPTH OF 6 INCHES,
4. EXCAVATION OF UP TO FIVE (5.0) FEET OF THE EXISTING IN-SITU FILL IN THE PROPOSED BUILDING AREA CAN BE EXPECTED, A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER SHOULD VERIFY THE DEPTH OF REMOVAL AT THE TIME OF CONSTRUCTION.
5. THE EXPOSED SUBGRADE SHOULD BE PROOF-ROLLED TO LOCATE ANY SOFT OR LOOSE AREAS IN ACCORDANCE WITH ITEM 216 OF TxDOT SPECIFICATIONS. SOILS THAT ARE OBSERVED TO RUT OR DEFLECT UNDER THE MOVING LOAD SHOULD BE UNDERCUT. THE PROOF ROLLING AND UNDERCUTTING ACTIVITIES SHOULD BE WITNESSED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER,
6. AFTER PROOFROLLING, THE UPPER 6 INCHES OF THE SUBGRADE SHOULD BE SCARIFIED, MOISTURE CONDITIONED TO A RANGE BETWEEN +1 TO +3 PERCENTAGE POINTS ABOVE THE OPTIMUM MOISTURE VALUE, AND COMPACTED TO AT LEAST 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-698.
7. AFTER SUBGRADE PREPERATION IS COMPLETE, THE PLACEMENT OF STRUCTURAL FILL MAY BEGIN. THE STRUCTURAL FILL MATERIALS SHOULD BE FREE OF ORGANICS OR OTHER DELETERIOUS MATERIALS AND SHOULD HAVE A MAXIMUM PARTICLE LESS THAN 2-INCHES. THE STRUCTURAL FILL MATERIAL SHOULD HAVE A LIQUID LIMIT LESS THAN 35 AND A PLASTICITY INDEX BETWEEN 6 AND 15. THE FILL SHOULD BE PLACED IN COMPACTION CONTROLLED 8 INCH HORIZONTAL LIFTS AND SHOULD BE COMPACTED WITHIN THE RANGE OF -2 TO +3 PERCENTAGE POINTS ABOVE THE OPTIMUM MOISTURE CONTENT VALUE, AND COMPACTED TO AT LEAST 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-698, EACH LIFT OF STRUCTURAL FILL SHOULD BE TESTED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER,
8. THE EXPOSED SUBGRADE SHALL BE PROTECTED AGAINST EXCESSIVE DRYING OR EXPOSURE TO MOISTURE, THE SITE SHALL BE GRADED TO PREVENT PONDING OF WATER WITHIN EXCAVATED AREAS. ACCUMULATED WATER SHALL BE REMOVED IMMEDIATELY. EXCAVATIONS FOR FOUNDATIONS SHOULD BE FILLED WITH CONCRETE BEFORE THE END OF THE WORKDAY OR SOONER IF NECESSARY TO PREVENT DETERIORATION OF THE BEARING SURFACE. IF DELAYS OCCUR, THE EXCAVATION SHOULD BE DEEPENED AS NECESSARY AND CLEANED, IN ORDER TO PROVIDE A FRESH BEARING SURFACE. IF MORE THAN 24 HOURS OF EXPOSURE OF THE BEARING SURFACE IS ANTICIPATED IN THE EXCAVATIONS, A MUD SLAB SHOULD BE USED TO PROTECT THE BEARING SURFACES. IF A MUD SLAB IS USED, THE FOUNDATION EXCAVATIONS SHOULD INITIALLY BE OVER-EXCAVATED BY APPROXIMATELY 4 INCHES AND A LEAN CONCRETE MUD SLAB OF APPROXIMATELY 4 INCHES IN THICKNESS IS PLACED IN THE BOTTOM OF THE EXCAVATIONS IMMEDIATELY FOLLOWING EXPOSURE OF THE BEARING SURFACE.
9. THIS PAD PREPARATION DOES NOT CONSTITUTE A COMPLETE SITE WORK SPECIFICATION. REFER TO THE GEOTECHNICAL REPORT FOR SPECIFIC INFORMATION NOT COVERED IN THIS PREPARATION.

FOUNDATION PLAN NOTES **C**

CONCRETE:
A. CONCRETE SHALL BE HARD ROCK CONC. (5 SACK CEMENT PER CU.YD. MIN.) AND MEET THE FOLLOWING MIN. ULTIMATE COMPRESSIVE STRENGTHS AT 28 DAYS:

LOCATION	MIN. STRENGTH 28 DAYS PSI	AGGREGATE SIZE - INCHESS	SLUMP INCHES	TOLERANCE
SLAB ON GRADE	(4000 DESIGN)	1" x #4	3-1/2"	±1/2"
FOUNDATIONS	(3000 DESIGN)	1" x #4	3-1/2"	±1/2"

B. CONCRETE MIX DESIGN AND TESTING SHALL MEET WITH THESE SPECS. CEMENT SHALL BE IN ACCORDANCE WITH ASTM C 150 TYPE II. VERIFY MIN. CONC. STRENGTH AND CEMENT TYPE.
C. REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60. STEEL SHALL BE KEPT CLEAN AND FREE OF RUST.
D. CONCRETE CURING SHALL BE IN ACCORDANCE WITH REQUIREMENTS OF ACI-318-95 SECTION 5.11 AND STANDARD PRACTICE FOR CURING CONCRETE REPORTED BY COMMITTEE 308.
E. ANCHOR BOLTS = A36 OR A307. USE 5/8" DIAMETER x 12" ANCHOR BOLTS (A.B.) AT 72" O.C. U.O.N. ANCHOR BOLTS SHALL BE TIED IN PLACE PRIOR TO PLACEMENT OF CONC.

SLAB:
A. DESIGN IS BASED UPON 4" THICK CONCRETE SLAB REINFORCED W/ WWF 6x6-W1.4x1.4 OR #4 BARS @ 18" O.C. EA. WAY. OVER 6 MIL VISQUEEN MEMBRANE, OVER 2" SAND BED, OVER 4" AGGREGATE BASE. OVER ENGINEERED SUBGRADE. MODIFY AS REQUIRED TO COMPLY WITH REQUIREMENTS OF SOILS REPORT.

MISCELLANEOUS:
A. DIMENSIONS NOTED ARE TO FACE OF CONCRETE. REFER TO DWG. A1.0 FOR DIMENSIONS TO FACE OF STUD AND OTHER DIMENSIONS NOT OTHERWISE NOTED.
B. DRAWINGS SHALL NOT BE SCALED. ALL DIMENSIONS AND FIT SHALL BE DETERMINED AND VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCEMENT OF WORK.
C. DETAILS NOT FULLY OR SPECIFICALLY SHOWN SHALL BE OF SAME NATURE AS OTHER SIMILAR CONDITIONS.
D. SEE PLUMB. DWGS. FOR PLUMB. LAYOUT DIMENSIONS, U.O.N.
E. SEE ELECT. DWGS. FOR ELECT. LAYOUT DIMENSIONS, U.O.N.
F. COORD. FOUNDATION AND SLAB LAYOUT WITH OTHER TRADES PRIOR TO POURING SLAB.

1. SLAB SHALL BE PITCHED ¼" FOR 5'-0" x 5'-0" SQUARE AT ALL FLOOR DRAINS U.O.N. REFER TO PLUMBING DRAWINGS FOR LOCATIONS.
2. PROVIDE HUB DRAIN (HD) UNLESS REQUIRED BY LOCAL CODE TO HAVE FLOOR SINK (FS).
3. INDICATES INSIDE SURFACE OF FOOTING. SEE SHEET S4.0.
4. HTT5 HOLDOWN ANCHOR. SEE 6/S4.0 FOR HOLDOWN EMBEDMENT DETAIL.
5. ANCHOR BOLTS LOCATED THROUGHOUT PERIMETER OF BUILDING SHALL BE PROVIDED AS REQUIRED PER THE "PLATE/ANCHOR BOLT" COLUMN OF THE "WALL SHEATHING AND SHEARWALL SCHEDULE." SEE D/S2.0.
6. HDU5 HOLDOWN ANCHOR AT EACH END OF SHEARWALL. SEE 6/S4.0 FOR HOLDOWN EMBEDMENT DETAIL.
7. HDU11 HOLDOWN ANCHOR AT EACH END OF SHEARWALL. SEE 6/S4.0 FOR HOLDOWN EMBEDMENT DETAIL.
8. HDU14 HOLDOWN ANCHOR AT EACH END OF INTERIOR SHEARWALL. SEE 6/S4.0 FOR HOLDOWN EMBEDMENT DETAIL. .
9. INTERIOR WALLS (TYP.) SEE SHEET S2.0
10. COLUMN HSS 6x6x¼ ON 3'-0'x3'-0'x12" THICK FOOTING

KEY NOTES **B**

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JOB: 16049

CONTRACT DATE:	4.3.17
BUILDING TYPE:	EXPLORER LG-54
PLAN VERSION:	NOVEMBER 2015
SITE NUMBER:	XXXXX
STORE NUMBER:	XXXXX

TACO BELL
LAKEPOINT
EMERSON, GA 30137

EXPLORER

FOUNDATION PLAN

S1.0

PLOT DATE: 08/23/17